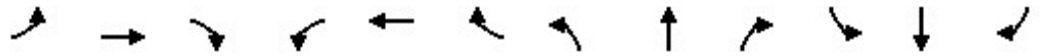


Queues

1: W Broad Street & Gaskins Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	703	523	318	51	734	493	249	871	12	224	906	313
v/c Ratio	1.46	0.65	0.53	0.42	1.48	0.93	0.63	0.39	0.01	0.60	0.42	0.37
Control Delay	254.8	45.3	7.9	60.5	262.1	40.5	55.3	23.5	0.0	55.2	24.4	3.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	254.8	45.3	7.9	60.5	262.1	40.5	55.3	23.5	0.0	55.2	24.4	3.9
Queue Length 50th (ft)	~363	188	0	36	~390	107	91	157	0	81	168	0
Queue Length 95th (ft)	#482	255	79	76	#513	#318	129	205	0	118	218	56
Internal Link Dist (ft)		695			440			948			564	
Turn Bay Length (ft)	550		550	270		270	550		950	425		430
Base Capacity (vph)	481	810	602	155	496	528	572	2218	811	790	2159	838
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.46	0.65	0.53	0.33	1.48	0.93	0.44	0.39	0.01	0.28	0.42	0.37

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM 6th Signalized Intersection Summary

## 1: W Broad Street & Gaskins Road

10/12/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	689	513	312	50	719	483	244	854	12	220	888	307
Future Volume (veh/h)	689	513	312	50	719	483	244	854	12	220	888	307
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1885	1870	1900	1900	1885	1885	1870
Adj Flow Rate, veh/h	703	523	0	51	734	493	249	871	12	224	906	0
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	2	2	2	2	1	2	0	0	1	1	2
Cap, veh/h	485	866		66	499	224	317	2336	725	295	2281	
Arrive On Green	0.14	0.24	0.00	0.04	0.14	0.14	0.09	0.45	0.45	0.08	0.44	0.00
Sat Flow, veh/h	3456	3554	1585	1781	3554	1594	3456	5187	1609	3483	5147	1585
Grp Volume(v), veh/h	703	523	0	51	734	493	249	871	12	224	906	0
Grp Sat Flow(s),veh/h/ln	1728	1777	1585	1781	1777	1594	1728	1729	1609	1742	1716	1585
Q Serve(g_s), s	16.0	14.9	0.0	3.2	16.0	16.0	8.0	12.6	0.5	7.2	13.6	0.0
Cycle Q Clear(g_c), s	16.0	14.9	0.0	3.2	16.0	16.0	8.0	12.6	0.5	7.2	13.6	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	485	866		66	499	224	317	2336	725	295	2281	
V/C Ratio(X)	1.45	0.60		0.78	1.47	2.20	0.78	0.37	0.02	0.76	0.40	
Avail Cap(c_a), veh/h	485	866		156	499	224	576	2336	725	794	2281	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	49.0	38.2	0.0	54.4	49.0	49.0	50.7	20.7	17.4	51.0	21.4	0.0
Incr Delay (d2), s/veh	213.6	1.2	0.0	13.3	223.0	555.9	3.2	0.5	0.0	3.0	0.5	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	21.3	6.6	0.0	1.7	22.5	40.9	3.6	5.2	0.2	3.2	5.5	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	262.6	39.4	0.0	67.8	272.0	604.9	53.9	21.2	17.4	54.0	22.0	0.0
LnGrp LOS	F	D		E	F	F	D	C	B	D	C	
Approach Vol, veh/h		1226	A		1278			1132			1130	A
Approach Delay, s/veh		167.4			392.2			28.3			28.3	
Approach LOS		F			F			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	15.5	56.5	21.0	21.0	14.7	57.3	9.2	32.8				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	19.0	42.0	16.0	16.0	26.0	35.0	10.0	22.0				
Max Q Clear Time (g_c+I1), s	10.0	15.6	18.0	18.0	9.2	14.6	5.2	16.9				
Green Ext Time (p_c), s	0.4	12.2	0.0	0.0	0.5	10.2	0.0	1.5				

### Intersection Summary

HCM 6th Ctrl Delay	161.7
HCM 6th LOS	F

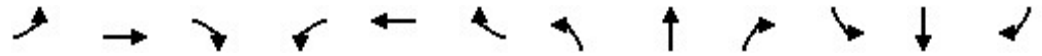
### Notes

Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.

# Queues

## 2: W Broad Street & Pemberton Road / Springfield Road

10/12/2021



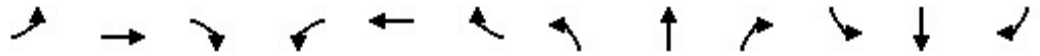
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	120	158	187	424	135	95	145	1346	359	98	1187	64
v/c Ratio	0.66	0.72	0.37	1.00	0.52	0.29	0.69	0.55	0.31	0.58	0.51	0.08
Control Delay	65.9	67.0	20.0	93.5	53.8	6.0	64.9	22.7	2.5	62.8	23.8	0.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	65.9	67.0	20.0	93.5	53.8	6.0	64.9	22.7	2.5	62.8	23.8	0.6
Queue Length 50th (ft)	85	112	61	162	92	0	103	256	14	70	228	0
Queue Length 95th (ft)	146	#186	119	#268	159	29	167	319	52	123	286	4
Internal Link Dist (ft)		380			538			1004			707	
Turn Bay Length (ft)	150		350	460			400			270		280
Base Capacity (vph)	219	245	544	425	259	327	261	2444	1154	219	2315	783
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.55	0.64	0.34	1.00	0.52	0.29	0.56	0.55	0.31	0.45	0.51	0.08

### Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 2: W Broad Street & Pemberton Road / Springfield Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	110	145	172	390	124	87	133	1238	330	90	1092	59
Future Volume (veh/h)	110	145	172	390	124	87	133	1238	330	90	1092	59
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1870	1870	1885	1885	1885	1856	1870	1885	1885	1870	1870
Adj Flow Rate, veh/h	120	158	187	424	135	95	145	1346	359	98	1187	64
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	1	2	2	1	1	1	3	2	1	1	2	2
Cap, veh/h	147	231	351	428	309	262	174	2558	997	123	2407	747
Arrive On Green	0.08	0.12	0.12	0.12	0.16	0.16	0.10	0.50	0.50	0.07	0.47	0.47
Sat Flow, veh/h	1795	1870	1585	3483	1885	1598	1767	5106	1598	1795	5106	1585
Grp Volume(v), veh/h	120	158	187	424	135	95	145	1346	359	98	1187	64
Grp Sat Flow(s),veh/h/ln	1795	1870	1585	1742	1885	1598	1767	1702	1598	1795	1702	1585
Q Serve(g_s), s	7.5	9.2	11.9	13.9	7.4	6.0	9.2	20.4	12.4	6.1	18.3	2.5
Cycle Q Clear(g_c), s	7.5	9.2	11.9	13.9	7.4	6.0	9.2	20.4	12.4	6.1	18.3	2.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	147	231	351	428	309	262	174	2558	997	123	2407	747
V/C Ratio(X)	0.81	0.69	0.53	0.99	0.44	0.36	0.84	0.53	0.36	0.79	0.49	0.09
Avail Cap(c_a), veh/h	220	246	364	428	309	262	264	2558	997	220	2407	747
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.5	47.9	39.2	49.9	42.9	42.4	50.5	19.3	10.4	52.3	20.7	16.6
Incr Delay (d2), s/veh	11.1	7.1	1.4	41.1	1.0	0.8	11.2	0.8	1.0	8.3	0.7	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.8	4.7	4.7	8.4	3.5	2.4	4.6	8.0	4.5	3.0	7.3	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	62.6	54.9	40.5	91.0	43.9	43.2	61.7	20.1	11.4	60.6	21.5	16.8
LnGrp LOS	E	D	D	F	D	D	E	C	B	E	C	B
Approach Vol, veh/h		465			654			1850			1349	
Approach Delay, s/veh		51.1			74.3			21.6			24.1	
Approach LOS		D			E			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	16.2	59.7	19.0	19.1	12.8	63.1	14.4	23.7				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	17.0	47.0	14.0	15.0	14.0	50.0	14.0	15.0				
Max Q Clear Time (g_c+I1), s	11.2	20.3	15.9	13.9	8.1	22.4	9.5	9.4				
Green Ext Time (p_c), s	0.1	16.7	0.0	0.2	0.1	21.2	0.1	0.5				
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay			33.6									
HCM 6th LOS			C									

Queues

3: W Broad Street & West End Drive / Commercial Ent.

10/12/2021



Lane Group	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	7	39	279	277	161	37	1180	172	60	1884
v/c Ratio	0.08	0.17	0.73	0.72	0.33	0.32	0.45	0.14	0.45	0.67
Control Delay	54.0	1.5	53.8	53.3	6.7	57.7	19.1	0.7	60.0	21.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	54.0	1.5	53.8	53.3	6.7	57.7	19.1	0.7	60.0	21.4
Queue Length 50th (ft)	5	0	205	203	0	26	206	0	43	388
Queue Length 95th (ft)	20	0	#354	#350	48	60	263	11	85	472
Internal Link Dist (ft)	349			285			640			671
Turn Bay Length (ft)					160	65		215	450	
Base Capacity (vph)	191	316	384	386	495	170	2625	1261	304	2809
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.04	0.12	0.73	0.72	0.33	0.22	0.45	0.14	0.20	0.67
























Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 3: W Broad Street & West End Drive / Commercial Ent.

10/12/2021

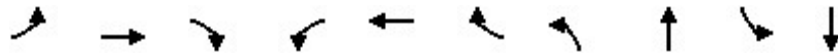
													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	5	2	36	509	8	150	34	1097	160	56	1729	23	
Future Volume (vph)	5	2	36	509	8	150	34	1097	160	56	1729	23	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0	5.0	5.0	5.0	5.0	6.0	5.0	5.0	6.0		
Lane Util. Factor		1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91		
Frbp, ped/bikes		1.00	0.98	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Frt		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00		
Flt Protected		0.97	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (prot)		1821	1533	1681	1689	1583	1770	5085	1568	1736	5125		
Flt Permitted		0.97	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (perm)		1821	1533	1681	1689	1583	1770	5085	1568	1736	5125		
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Adj. Flow (vph)	5	2	39	547	9	161	37	1180	172	60	1859	25	
RTOR Reduction (vph)	0	0	38	0	0	124	0	0	48	0	1	0	
Lane Group Flow (vph)	0	7	1	279	277	37	37	1180	124	60	1883	0	
Confl. Peds. (#/hr)			1	1									
Heavy Vehicles (%)	1%	0%	3%	2%	0%	2%	2%	2%	3%	4%	1%	2%	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	pm+ov	Prot	NA		
Protected Phases	3	3		4	4		1	6	4	5	2		
Permitted Phases			3			4			6				
Actuated Green, G (s)		3.3	3.3	26.1	26.1	26.1	5.2	55.8	81.9	7.8	58.4		
Effective Green, g (s)		3.3	3.3	26.1	26.1	26.1	5.2	55.8	81.9	7.8	58.4		
Actuated g/C Ratio		0.03	0.03	0.23	0.23	0.23	0.05	0.49	0.72	0.07	0.51		
Clearance Time (s)		5.0	5.0	5.0	5.0	5.0	5.0	6.0	5.0	5.0	6.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0	2.5	4.0	2.0	2.5	4.0		
Lane Grp Cap (vph)		52	44	384	386	362	80	2488	1126	118	2625		
v/s Ratio Prot		c0.00		c0.17	0.16		0.02	0.23	0.03	c0.03	c0.37		
v/s Ratio Perm			0.00			0.02			0.05				
v/c Ratio		0.13	0.03	0.73	0.72	0.10	0.46	0.47	0.11	0.51	0.72		
Uniform Delay, d1		54.0	53.8	40.6	40.5	34.7	53.0	19.3	4.9	51.2	21.4		
Progression Factor		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2		0.4	0.1	5.7	5.2	0.0	3.1	0.7	0.0	2.5	1.7		
Delay (s)		54.4	53.9	46.4	45.8	34.7	56.1	20.0	4.9	53.7	23.2		
Level of Service		D	D	D	D	C	E	B	A	D	C		
Approach Delay (s)		54.0			43.5			19.1			24.1		
Approach LOS		D			D			B			C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			26.1		HCM 2000 Level of Service					C			
HCM 2000 Volume to Capacity ratio			0.70										
Actuated Cycle Length (s)			114.0		Sum of lost time (s)					21.0			
Intersection Capacity Utilization			72.4%		ICU Level of Service					C			
Analysis Period (min)			15										

c Critical Lane Group

Queues

4: W Broad Street & N Parham Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	37	689	89	93	686	405	194	879	395	1439
v/c Ratio	0.34	0.90	0.20	0.58	0.69	0.55	0.80	0.55	1.16	0.76
Control Delay	59.2	60.4	4.0	63.4	42.4	6.8	71.3	32.6	140.6	35.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	59.2	60.4	4.0	63.4	42.4	6.8	71.3	32.6	140.6	35.1
Queue Length 50th (ft)	27	261	0	67	248	0	138	187	~343	341
Queue Length 95th (ft)	60	#421	21	117	#357	85	#231	231	#535	408
Internal Link Dist (ft)		487			952			657		412
Turn Bay Length (ft)	230		55	475		875	475		275	
Base Capacity (vph)	217	764	436	248	988	737	279	1601	341	1882
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.90	0.20	0.38	0.69	0.55	0.70	0.55	1.16	0.76

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

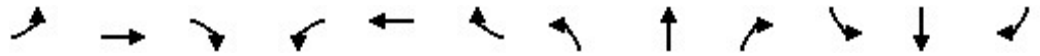
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 4: W Broad Street & N Parham Road

10/12/2021



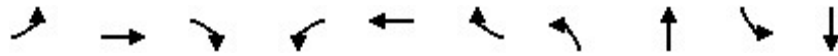
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑↑		↘	↑↑↑	
Traffic Volume (veh/h)	36	668	86	90	665	393	188	725	128	383	1353	43
Future Volume (veh/h)	36	668	86	90	665	393	188	725	128	383	1353	43
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1856	1870	1856	1870	1870	1885	1885	1870	1870	1870
Adj Flow Rate, veh/h	37	689	89	93	686	405	194	747	132	395	1395	44
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	3	2	3	2	2	1	1	2	2	2
Cap, veh/h	54	592	262	117	713	321	223	1720	301	344	2330	73
Arrive On Green	0.03	0.17	0.17	0.07	0.20	0.20	0.13	0.39	0.39	0.19	0.46	0.46
Sat Flow, veh/h	1781	3554	1572	1781	3526	1585	1781	4406	771	1781	5085	160
Grp Volume(v), veh/h	37	689	89	93	686	405	194	580	299	395	934	505
Grp Sat Flow(s),veh/h/ln	1781	1777	1572	1781	1763	1585	1781	1716	1746	1781	1702	1841
Q Serve(g_s), s	2.3	19.0	5.7	5.9	22.0	23.1	12.2	14.1	14.3	22.0	23.3	23.3
Cycle Q Clear(g_c), s	2.3	19.0	5.7	5.9	22.0	23.1	12.2	14.1	14.3	22.0	23.3	23.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.44	1.00		0.09
Lane Grp Cap(c), veh/h	54	592	262	117	713	321	223	1339	682	344	1560	844
V/C Ratio(X)	0.69	1.16	0.34	0.79	0.96	1.26	0.87	0.43	0.44	1.15	0.60	0.60
Avail Cap(c_a), veh/h	219	592	262	250	713	321	281	1339	682	344	1560	844
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.7	47.5	42.0	52.5	45.0	45.5	49.0	25.5	25.6	46.0	23.1	23.1
Incr Delay (d2), s/veh	5.6	91.0	2.8	4.5	25.3	141.1	17.9	1.0	2.0	95.4	1.7	3.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	15.8	2.4	2.8	12.0	21.6	6.5	5.9	6.3	18.8	9.6	10.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.4	138.5	44.7	57.0	70.4	186.6	66.8	26.5	27.6	141.4	24.8	26.2
LnGrp LOS	E	F	D	E	E	F	E	C	C	F	C	C
Approach Vol, veh/h		815			1184			1073			1834	
Approach Delay, s/veh		124.7			109.1			34.1			50.3	
Approach LOS		F			F			C			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	19.3	58.2	8.5	28.1	27.0	50.5	12.5	24.0				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	18.0	40.0	14.0	21.0	22.0	36.0	16.0	19.0				
Max Q Clear Time (g_c+I1), s	14.2	25.3	4.3	25.1	24.0	16.3	7.9	21.0				
Green Ext Time (p_c), s	0.1	12.7	0.0	0.0	0.0	11.5	0.1	0.0				
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay			73.3									
HCM 6th LOS			E									



Queues

5: W Broad Street & Hungary Spring Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	70	258	148	281	239	82	93	1289	140	1434
v/c Ratio	0.29	1.02	0.28	1.04	0.84	0.22	0.57	0.69	0.71	0.73
Control Delay	48.0	110.9	12.6	114.0	73.4	1.3	62.1	30.6	68.8	32.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.0	110.9	12.6	114.0	73.4	1.3	62.1	30.6	68.8	32.4
Queue Length 50th (ft)	47	~197	26	~234	174	0	66	272	100	319
Queue Length 95th (ft)	92	#364	74	#403	#320	0	117	330	166	396
Internal Link Dist (ft)		431			703			305		418
Turn Bay Length (ft)	225		350	325			275		175	
Base Capacity (vph)	240	253	574	270	283	377	222	1862	227	1973
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	1.02	0.26	1.04	0.84	0.22	0.42	0.69	0.62	0.73

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

























# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 5: W Broad Street & Hungary Spring Road

10/12/2021

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	68	250	144	273	232	80	90	921	330	136	1345	46	
Future Volume (vph)	68	250	144	273	232	80	90	921	330	136	1345	46	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	6.5	6.5	6.5	6.5	6.5	6.5	6.5	6.8		6.5	6.5		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91		1.00	0.91		
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.96		1.00	1.00		
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1752	4871		1787	5062		
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (perm)	1770	1863	1583	1770	1863	1583	1752	4871		1787	5062		
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	70	258	148	281	239	82	93	949	340	140	1387	47	
RTOR Reduction (vph)	0	0	76	0	0	69	0	56	0	0	3	0	
Lane Group Flow (vph)	70	258	72	281	239	13	93	1233	0	140	1431	0	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	3%	2%	3%	1%	2%	1%	
Turn Type	Split	NA	pm+ov	Split	NA	Perm	Prot	NA		Prot	NA		
Protected Phases	4	4		3	3		1	6		5	2		
Permitted Phases			4			3							
Actuated Green, G (s)	15.5	15.5	26.2	17.4	17.4	17.4	10.7	42.3		12.5	44.4		
Effective Green, g (s)	15.5	15.5	26.2	17.4	17.4	17.4	10.7	42.3		12.5	44.4		
Actuated g/C Ratio	0.14	0.14	0.23	0.15	0.15	0.15	0.09	0.37		0.11	0.39		
Clearance Time (s)	6.5	6.5	6.5	6.5	6.5	6.5	6.5	6.8		6.5	6.5		
Vehicle Extension (s)	3.0	3.0	2.5	3.0	3.0	3.0	2.5	4.0		2.5	4.0		
Lane Grp Cap (vph)	240	253	454	270	284	241	164	1807		195	1971		
v/s Ratio Prot	0.04	c0.14	0.01	c0.16	0.13		0.05	0.25		c0.08	c0.28		
v/s Ratio Perm			0.03			0.01							
v/c Ratio	0.29	1.02	0.16	1.04	0.84	0.05	0.57	0.68		0.72	0.73		
Uniform Delay, d1	44.3	49.2	35.1	48.3	47.0	41.3	49.4	30.2		49.0	29.6		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00		
Incremental Delay, d2	0.7	61.8	0.1	65.8	19.6	0.1	3.6	2.1		11.1	2.4		
Delay (s)	45.0	111.0	35.2	114.1	66.6	41.3	53.0	32.3		60.2	32.0		
Level of Service	D	F	D	F	E	D	D	C		E	C		
Approach Delay (s)		77.7			85.3			33.7			34.5		
Approach LOS		E			F			C			C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			46.9		HCM 2000 Level of Service						D		
HCM 2000 Volume to Capacity ratio			0.86										
Actuated Cycle Length (s)			114.0		Sum of lost time (s)					26.3			
Intersection Capacity Utilization			82.9%		ICU Level of Service					E			
Analysis Period (min)			15										
c Critical Lane Group													

Queues

6: W Broad Street & Bethlehem Road / Ent. To Volvo

10/12/2021



Lane Group	EBT	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	6	106	95	17	1377	120	1728
v/c Ratio	0.04	0.70	0.37	0.18	0.43	0.65	0.46
Control Delay	0.5	72.0	13.3	55.3	11.1	64.5	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	0.5	72.0	13.3	55.3	11.1	64.5	7.0
Queue Length 50th (ft)	0	75	0	12	175	85	127
Queue Length 95th (ft)	0	#150	49	36	221	145	249
Internal Link Dist (ft)	239	270			271		254
Turn Bay Length (ft)			95	200		175	
Base Capacity (vph)	254	172	282	177	3239	232	3758
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.62	0.34	0.10	0.43	0.52	0.46

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 6: W Broad Street & Bethlehem Road / Ent. To Volvo

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕	↕	↑↑↑		↕	↑↑↑	
Traffic Volume (veh/h)	4	0	2	99	2	90	16	1268	40	114	1626	15
Future Volume (veh/h)	4	0	2	99	2	90	16	1268	40	114	1626	15
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1856	1796	1856	1856	1870	1856	1856
Adj Flow Rate, veh/h	4	0	2	104	2	95	17	1335	42	120	1712	16
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	0	0	0	0	0	3	7	3	3	2	3	3
Cap, veh/h	71	10	14	192	2	149	31	3440	108	147	3863	36
Arrive On Green	0.10	0.00	0.10	0.10	0.10	0.10	0.02	0.68	0.68	0.08	0.75	0.75
Sat Flow, veh/h	188	110	149	1359	26	1572	1711	5045	159	1781	5175	48
Grp Volume(v), veh/h	6	0	0	106	0	95	17	894	483	120	1117	611
Grp Sat Flow(s),veh/h/ln	448	0	0	1385	0	1572	1711	1689	1827	1781	1689	1847
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	6.6	1.1	13.0	13.0	7.6	14.3	14.3
Cycle Q Clear(g_c), s	8.7	0.0	0.0	8.6	0.0	6.6	1.1	13.0	13.0	7.6	14.3	14.3
Prop In Lane	0.67		0.33	0.98		1.00	1.00		0.09	1.00		0.03
Lane Grp Cap(c), veh/h	95	0	0	194	0	149	31	2303	1246	147	2521	1378
V/C Ratio(X)	0.06	0.00	0.00	0.55	0.00	0.64	0.54	0.39	0.39	0.81	0.44	0.44
Avail Cap(c_a), veh/h	134	0	0	234	0	193	180	2303	1246	234	2521	1378
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	47.1	0.0	0.0	50.6	0.0	49.7	55.5	7.8	7.8	51.4	5.5	5.5
Incr Delay (d2), s/veh	0.2	0.0	0.0	1.8	0.0	3.3	10.5	0.5	0.9	8.8	0.6	1.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.0	0.0	3.1	0.0	2.7	0.6	4.5	5.0	3.7	4.5	5.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	47.3	0.0	0.0	52.4	0.0	53.0	66.0	8.3	8.8	60.2	6.0	6.5
LnGrp LOS	D	A	A	D	A	D	E	A	A	E	A	A
Approach Vol, veh/h		6			201			1394			1848	
Approach Delay, s/veh		47.3			52.7			9.2			9.7	
Approach LOS		D			D			A			A	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	7.1	91.1		15.8	14.4	83.7		15.8				
Change Period (Y+Rc), s	5.0	6.0		5.0	5.0	6.0		5.0				
Max Green Setting (Gmax), s	12.0	72.0		14.0	15.0	69.0		14.0				
Max Q Clear Time (g_c+I1), s	3.1	16.3		10.6	9.6	15.0		10.7				
Green Ext Time (p_c), s	0.0	30.1		0.2	0.1	20.9		0.0				

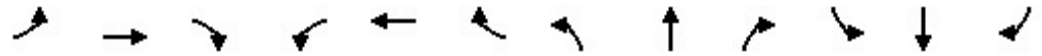
Intersection Summary

HCM 6th Ctrl Delay	12.1
HCM 6th LOS	B

Queues

7: W Broad Street & Glenside Drive

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	284	596	69	198	662	189	28	1442	235	156	1232	719
v/c Ratio	0.97	0.97	0.04	0.71	1.20	0.32	0.26	0.85	0.28	0.70	0.79	0.69
Control Delay	92.3	76.8	0.1	60.5	147.2	17.3	56.7	41.6	5.1	64.1	33.2	8.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	92.3	76.8	0.1	60.5	147.2	17.3	56.7	41.6	5.1	64.1	33.2	8.3
Queue Length 50th (ft)	229	241	0	139	~310	58	20	359	23	111	426	47
Queue Length 95th (ft)	#418	#363	0	#237	#428	114	49	#444	46	179	#577	194
Internal Link Dist (ft)		738			586			849			590	
Turn Bay Length (ft)	650		410			230	775		425	485		
Base Capacity (vph)	293	613	1568	279	553	618	155	1691	841	258	1561	1043
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.97	0.97	0.04	0.71	1.20	0.31	0.18	0.85	0.28	0.60	0.79	0.69

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 7: W Broad Street & Glenside Drive

10/12/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	368	486	67	192	642	183	27	1399	228	151	1195	697	
Future Volume (vph)	368	486	67	192	642	183	27	1399	228	151	1195	697	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.0	5.0	4.0	6.0	6.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0	
Lane Util. Factor	0.91	0.91	1.00	1.00	0.95	1.00	1.00	0.91	1.00	1.00	0.95	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected	0.95	0.99	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	1595	3331	1568	1770	3505	1583	1770	5036	1583	1736	3539	1583	
Flt Permitted	0.95	0.99	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (perm)	1595	3331	1568	1770	3505	1583	1770	5036	1583	1736	3539	1583	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	379	501	69	198	662	189	28	1442	235	156	1232	719	
RTOR Reduction (vph)	0	0	0	0	0	55	0	0	60	0	0	356	
Lane Group Flow (vph)	284	596	69	198	662	134	28	1442	175	156	1232	363	
Heavy Vehicles (%)	3%	3%	3%	2%	3%	2%	2%	3%	2%	4%	2%	2%	
Turn Type	Split	NA	Free	Split	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	Perm	
Protected Phases	3	3		4	4	5	1	6	4	5	2		
Permitted Phases			Free			4			6			2	
Actuated Green, G (s)	21.0	21.0	114.0	18.0	18.0	32.7	4.7	38.3	56.3	14.7	48.3	48.3	
Effective Green, g (s)	21.0	21.0	114.0	18.0	18.0	32.7	4.7	38.3	56.3	14.7	48.3	48.3	
Actuated g/C Ratio	0.18	0.18	1.00	0.16	0.16	0.29	0.04	0.34	0.49	0.13	0.42	0.42	
Clearance Time (s)	5.0	5.0		6.0	6.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0	
Vehicle Extension (s)	2.5	2.5		2.5	2.5	2.5	2.5	5.0	2.5	2.5	5.0	5.0	
Lane Grp Cap (vph)	293	613	1568	279	553	454	72	1691	781	223	1499	670	
v/s Ratio Prot	0.18	c0.18		0.11	c0.19	0.04	0.02	0.29	0.04	c0.09	c0.35		
v/s Ratio Perm			0.04			0.05			0.08			0.23	
v/c Ratio	0.97	0.97	0.04	0.71	1.20	0.30	0.39	0.85	0.22	0.70	0.82	0.54	
Uniform Delay, d1	46.2	46.2	0.0	45.5	48.0	31.7	53.3	35.2	16.4	47.5	29.0	24.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	43.6	29.3	0.1	7.5	105.3	0.3	2.5	5.7	0.1	8.5	5.2	3.1	
Delay (s)	89.8	75.5	0.1	53.0	153.3	31.9	55.8	40.9	16.5	56.1	34.2	27.7	
Level of Service	F	E	A	D	F	C	E	D	B	E	C	C	
Approach Delay (s)		74.3			112.5			37.8			33.6		
Approach LOS		E			F			D			C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			55.7									HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio			0.94										
Actuated Cycle Length (s)			114.0									Sum of lost time (s)	22.0
Intersection Capacity Utilization			89.4%									ICU Level of Service	E
Analysis Period (min)			15										
c Critical Lane Group													

Queues

8: Staples Mill Road & Hungary Road

10/12/2021



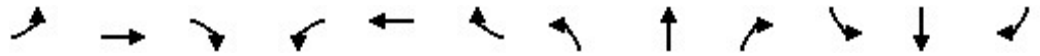
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	76	376	595	8	559	358	566	10	67	1180	151
v/c Ratio	0.55	0.35	0.65	0.11	0.80	0.73	0.32	0.01	0.54	0.79	0.19
Control Delay	76.3	38.9	15.6	67.3	60.7	65.7	23.3	0.0	77.3	41.0	3.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	76.3	38.9	15.6	67.3	60.7	65.7	23.3	0.0	77.3	41.0	3.9
Queue Length 50th (ft)	68	137	196	7	252	162	160	0	60	484	0
Queue Length 95th (ft)	120	185	298	26	301	207	252	0	109	#780	38
Internal Link Dist (ft)		434			495		535			418	
Turn Bay Length (ft)	275		230	200		315		115	340		240
Base Capacity (vph)	322	1134	957	193	824	607	1756	769	159	1499	776
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.33	0.62	0.04	0.68	0.59	0.32	0.01	0.42	0.79	0.19

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 8: Staples Mill Road & Hungary Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↙	↕	↘	↙	↕	↘	↙	↕	↘	↙	↕	↘
Traffic Volume (veh/h)	71	350	553	7	457	63	333	526	9	62	1097	140
Future Volume (veh/h)	71	350	553	7	457	63	333	526	9	62	1097	140
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1885	1870	1900	1841	1841	1841	1826	1648	1781	1870	1885
Adj Flow Rate, veh/h	76	376	595	8	491	68	358	566	0	67	1180	0
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	1	2	0	4	4	4	5	17	8	2	1
Cap, veh/h	97	1100	681	17	812	112	416	1678		84	1461	
Arrive On Green	0.05	0.31	0.31	0.01	0.26	0.26	0.12	0.48	0.00	0.05	0.41	0.00
Sat Flow, veh/h	1810	3582	1585	1810	3087	426	3401	3469	1397	1697	3554	1598
Grp Volume(v), veh/h	76	376	595	8	277	282	358	566	0	67	1180	0
Grp Sat Flow(s),veh/h/ln	1810	1791	1585	1810	1749	1764	1700	1735	1397	1697	1777	1598
Q Serve(g_s), s	5.8	11.4	43.0	0.6	19.4	19.6	14.5	14.1	0.0	5.5	41.0	0.0
Cycle Q Clear(g_c), s	5.8	11.4	43.0	0.6	19.4	19.6	14.5	14.1	0.0	5.5	41.0	0.0
Prop In Lane	1.00		1.00	1.00		0.24	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	97	1100	681	17	460	464	416	1678		84	1461	
V/C Ratio(X)	0.78	0.34	0.87	0.46	0.60	0.61	0.86	0.34		0.80	0.81	
Avail Cap(c_a), veh/h	323	1100	681	194	460	464	607	1678		158	1461	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	65.5	37.5	36.5	69.0	45.2	45.2	60.3	22.3	0.0	65.8	36.4	0.0
Incr Delay (d2), s/veh	9.8	0.2	12.1	13.6	2.2	2.3	7.4	0.5	0.0	11.8	4.9	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.0	5.1	20.7	0.4	8.8	8.9	6.7	5.9	0.0	2.7	18.7	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	75.3	37.7	48.6	82.6	47.4	47.5	67.7	22.8	0.0	77.6	41.3	0.0
LnGrp LOS	E	D	D	F	D	D	E	C		E	D	
Approach Vol, veh/h		1047			567			924	A		1247	A
Approach Delay, s/veh		46.6			47.9			40.2			43.2	
Approach LOS		D			D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.0	73.7	6.3	48.0	22.1	63.5	12.5	41.8				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	13.0	48.0	15.0	43.0	25.0	36.0	25.0	33.0				
Max Q Clear Time (g_c+I1), s	7.5	16.1	2.6	45.0	16.5	43.0	7.8	21.6				
Green Ext Time (p_c), s	0.0	8.0	0.0	0.0	0.7	0.0	0.1	2.6				

Intersection Summary

HCM 6th Ctrl Delay	44.1
HCM 6th LOS	D

Notes

Unsignalized Delay for [NBR, SBR] is excluded from calculations of the approach delay and intersection delay.



Queues

9: Staples Mill Road & Hungary Spring Road

10/12/2021


























Lane Group	EBL	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	211	102	143	283	16	73	638	31	28	1267	481
v/c Ratio	0.59	0.28	0.35	0.67	0.05	0.49	0.33	0.03	0.28	0.72	0.50
Control Delay	56.5	49.2	9.1	56.9	0.3	61.4	16.7	0.1	58.3	27.5	8.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.5	49.2	9.1	56.9	0.3	61.4	16.7	0.1	58.3	27.5	8.1
Queue Length 50th (ft)	78	37	6	107	0	53	143	0	20	380	54
Queue Length 95th (ft)	115	64	54	150	0	99	217	0	50	552	167
Internal Link Dist (ft)		599		369			448			430	
Turn Bay Length (ft)	340		340		80	210			280		215
Base Capacity (vph)	508	529	475	512	349	233	1906	954	189	1756	960
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.19	0.30	0.55	0.05	0.31	0.33	0.03	0.15	0.72	0.50

Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 9: Staples Mill Road & Hungary Spring Road

10/12/2021

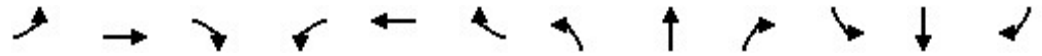
												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	196	95	133	95	168	15	68	593	29	26	1178	447
Future Volume (vph)	196	95	133	95	168	15	68	593	29	26	1178	447
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	5.0		6.0	6.0	5.0	6.0	6.0	5.0	6.0	6.0
Lane Util. Factor	0.97	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frpb, ped/bikes	1.00	1.00	0.99		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00		0.98	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3467	3610	1556		3499	1615	1805	3438	1615	1687	3539	1583
Flt Permitted	0.95	1.00	1.00		0.98	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3467	3610	1556		3499	1615	1805	3438	1615	1687	3539	1583
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	211	102	143	102	181	16	73	638	31	28	1267	481
RTOR Reduction (vph)	0	0	108	0	0	14	0	0	14	0	0	174
Lane Group Flow (vph)	211	102	35	0	283	2	73	638	17	28	1267	307
Confl. Peds. (#/hr)			1	1								
Heavy Vehicles (%)	1%	0%	3%	2%	1%	0%	0%	5%	0%	7%	2%	2%
Turn Type	Split	NA	pm+ov	Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	4	4	5	3	3		5	2		1	6	
Permitted Phases			4			3			2			6
Actuated Green, G (s)	11.9	11.9	21.5		14.0	14.0	9.6	62.3	62.3	4.8	57.5	57.5
Effective Green, g (s)	11.9	11.9	21.5		14.0	14.0	9.6	62.3	62.3	4.8	57.5	57.5
Actuated g/C Ratio	0.10	0.10	0.19		0.12	0.12	0.08	0.54	0.54	0.04	0.50	0.50
Clearance Time (s)	6.0	6.0	5.0		6.0	6.0	5.0	6.0	6.0	5.0	6.0	6.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5	2.5	6.0	6.0	2.5	6.0	6.0
Lane Grp Cap (vph)	355	370	288		422	194	149	1846	867	69	1754	784
v/s Ratio Prot	c0.06	0.03	0.01		c0.08		c0.04	c0.19		0.02	c0.36	
v/s Ratio Perm			0.01			0.00			0.01			0.19
v/c Ratio	0.59	0.28	0.12		0.67	0.01	0.49	0.35	0.02	0.41	0.72	0.39
Uniform Delay, d1	49.7	48.1	39.4		48.8	44.9	50.9	15.3	12.6	54.2	23.0	18.3
Progression Factor	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	2.2	0.3	0.1		3.8	0.0	1.8	0.5	0.0	2.8	2.6	1.5
Delay (s)	52.0	48.4	39.5		52.6	44.9	52.7	15.8	12.6	57.0	25.6	19.8
Level of Service	D	D	D		D	D	D	B	B	E	C	B
Approach Delay (s)		47.3			52.2			19.3			24.5	
Approach LOS		D			D			B			C	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			29.0		HCM 2000 Level of Service				C			
HCM 2000 Volume to Capacity ratio			0.67									
Actuated Cycle Length (s)			116.0		Sum of lost time (s)				23.0			
Intersection Capacity Utilization			69.1%		ICU Level of Service				C			
Analysis Period (min)			15									

c Critical Lane Group

Queues

10: Staples Mill Road & Staples Mill Square Shopping Center

10/12/2021



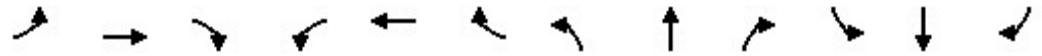
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	15	15	117	22	23	73	90	637	71	88	1321	96
v/c Ratio	0.15	0.14	0.30	0.24	0.24	0.29	0.41	0.31	0.07	0.58	0.63	0.10
Control Delay	54.4	54.1	9.1	58.0	58.0	2.9	57.1	14.4	0.1	65.5	18.4	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	54.4	54.1	9.1	58.0	58.0	2.9	57.1	14.4	0.1	65.5	18.4	0.2
Queue Length 50th (ft)	11	11	1	16	17	0	34	128	0	64	325	0
Queue Length 95th (ft)	33	33	48	45	46	0	59	199	0	116	474	0
Internal Link Dist (ft)		193			125			406			259	
Turn Bay Length (ft)	250		130	175		175	350			215		615
Base Capacity (vph)	158	163	385	140	144	290	299	2051	1003	172	2099	998
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.09	0.30	0.16	0.16	0.25	0.30	0.31	0.07	0.51	0.63	0.10

Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 10: Staples Mill Road & Staples Mill Square Shopping Center

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	20	8	110	37	6	69	85	599	67	83	1242	90
Future Volume (vph)	20	8	110	37	6	69	85	599	67	83	1242	90
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.3	7.3	7.3	8.0	8.0	8.0	7.6	5.9	5.9	7.8	5.9	5.9
Lane Util. Factor	0.95	0.95	1.00	0.95	0.95	1.00	0.97	0.95	1.00	1.00	0.95	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	0.98	1.00	0.95	0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1715	1770	1583	1633	1679	1583	3335	3438	1553	1805	3539	1553
Flt Permitted	0.95	0.98	1.00	0.95	0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1715	1770	1583	1633	1679	1583	3335	3438	1553	1805	3539	1553
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	21	9	117	39	6	73	90	637	71	88	1321	96
RTOR Reduction (vph)	0	0	93	0	0	70	0	0	31	0	0	40
Lane Group Flow (vph)	15	15	24	22	23	3	90	637	40	88	1321	56
Heavy Vehicles (%)	0%	0%	2%	5%	0%	2%	5%	5%	4%	0%	2%	4%
Turn Type	Split	NA	pt+ov	Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	4	4	4 5	3	3		5	2		1	6	
Permitted Phases						3			2			6
Actuated Green, G (s)	6.8	6.8	21.8	5.5	5.5	5.5	7.7	66.1	66.1	8.6	67.2	67.2
Effective Green, g (s)	6.8	6.8	21.8	5.5	5.5	5.5	7.7	66.1	66.1	8.6	67.2	67.2
Actuated g/C Ratio	0.06	0.06	0.19	0.05	0.05	0.05	0.07	0.57	0.57	0.07	0.58	0.58
Clearance Time (s)	7.3	7.3		8.0	8.0	8.0	7.6	5.9	5.9	7.8	5.9	5.9
Vehicle Extension (s)	2.0	2.0		2.0	2.0	2.0	2.0	6.0	6.0	2.0	6.0	6.0
Lane Grp Cap (vph)	100	103	297	77	79	75	221	1959	884	133	2050	899
v/s Ratio Prot	c0.01	0.01	0.01	0.01	c0.01		0.03	0.19		c0.05	c0.37	
v/s Ratio Perm						0.00			0.03			0.04
v/c Ratio	0.15	0.15	0.08	0.29	0.29	0.05	0.41	0.33	0.05	0.66	0.64	0.06
Uniform Delay, d1	51.9	51.8	38.8	53.4	53.4	52.7	52.0	13.2	11.0	52.3	16.4	10.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.3	0.2	0.0	0.7	0.7	0.1	0.4	0.4	0.1	9.2	1.6	0.1
Delay (s)	52.1	52.1	38.9	54.1	54.1	52.8	52.4	13.6	11.1	61.5	18.0	10.8
Level of Service	D	D	D	D	D	D	D	B	B	E	B	B
Approach Delay (s)		41.6			53.3			17.8			20.0	
Approach LOS		D			D			B			C	

### Intersection Summary

HCM 2000 Control Delay	22.1	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.59		
Actuated Cycle Length (s)	116.0	Sum of lost time (s)	29.0
Intersection Capacity Utilization	65.1%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

Queues

11: Staples Mill Road & Old Staples Mill Road / Lucas Road

10/12/2021
























Lane Group	EBT	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	117	339	52	88	771	107	100	1415
v/c Ratio	0.57	0.88	0.11	0.53	0.51	0.15	0.76	0.89
Control Delay	27.4	67.9	0.5	61.5	25.3	3.9	87.0	39.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.4	67.9	0.5	61.5	25.3	3.9	87.0	39.8
Queue Length 50th (ft)	20	236	0	64	221	0	75	538
Queue Length 95th (ft)	76	#416	0	114	281	31	#168	#756
Internal Link Dist (ft)	151	218			318			355
Turn Bay Length (ft)			50	150		450	250	
Base Capacity (vph)	242	396	486	248	1516	728	134	1587
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.48	0.86	0.11	0.35	0.51	0.15	0.75	0.89

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 11: Staples Mill Road & Old Staples Mill Road / Lucas Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	25	83	305	6	48	81	709	98	92	1302	0
Future Volume (vph)	0	25	83	305	6	48	81	709	98	92	1302	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0	
Lane Util. Factor		1.00			1.00	1.00	1.00	0.95	1.00	1.00	0.95	
Frt		0.90			1.00	0.85	1.00	1.00	0.85	1.00	1.00	
Flt Protected		1.00			0.95	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)		1703			1695	1524	1805	3438	1509	1671	3539	
Flt Permitted		1.00			0.95	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)		1703			1695	1524	1805	3438	1509	1671	3539	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	27	90	332	7	52	88	771	107	100	1415	0
RTOR Reduction (vph)	0	83	0	0	0	40	0	0	60	0	0	0
Lane Group Flow (vph)	0	34	0	0	339	12	88	771	47	100	1415	0
Heavy Vehicles (%)	0%	0%	0%	7%	0%	6%	0%	5%	7%	8%	2%	0%
Turn Type		NA		Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	3	3		4	4		5	2		1	6	
Permitted Phases						4			2			6
Actuated Green, G (s)		8.4			26.3	26.3	9.2	51.2	51.2	9.1	51.1	
Effective Green, g (s)		8.4			26.3	26.3	9.2	51.2	51.2	9.1	51.1	
Actuated g/C Ratio		0.07			0.23	0.23	0.08	0.44	0.44	0.08	0.44	
Clearance Time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0	
Vehicle Extension (s)		3.0			3.0	3.0	2.5	4.5	4.5	2.5	4.5	
Lane Grp Cap (vph)		123			384	345	143	1517	666	131	1558	
v/s Ratio Prot		c0.02			c0.20		0.05	0.22		c0.06	c0.40	
v/s Ratio Perm						0.01			0.03			
v/c Ratio		0.27			0.88	0.03	0.62	0.51	0.07	0.76	0.91	
Uniform Delay, d1		50.9			43.4	35.0	51.7	23.3	18.7	52.4	30.3	
Progression Factor		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2		1.2			20.5	0.0	6.6	1.2	0.2	21.9	9.3	
Delay (s)		52.1			63.9	35.0	58.2	24.6	18.9	74.3	39.6	
Level of Service		D			E	C	E	C	B	E	D	
Approach Delay (s)		52.1			60.1			27.0			41.9	
Approach LOS		D			E			C			D	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			39.8		HCM 2000 Level of Service					D		
HCM 2000 Volume to Capacity ratio			0.83									
Actuated Cycle Length (s)			116.0	Sum of lost time (s)					21.0			
Intersection Capacity Utilization			79.0%	ICU Level of Service					D			
Analysis Period (min)			15									
c Critical Lane Group												

Queues

12: Staples Mill Road & E Parham Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	111	638	290	470	703	141	351	763	338	278	1576
v/c Ratio	0.49	0.87	0.43	1.09	0.74	0.20	0.86	0.61	0.38	0.73	0.89
Control Delay	60.0	58.1	17.2	117.2	43.7	10.4	71.3	33.2	13.4	61.6	42.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.0	58.1	17.2	117.2	43.7	10.4	71.3	33.2	13.4	61.6	42.3
Queue Length 50th (ft)	41	244	88	~204	253	28	134	247	109	104	401
Queue Length 95th (ft)	72	#340	164	#309	323	68	#211	314	176	150	468
Internal Link Dist (ft)		473			499			459			344
Turn Bay Length (ft)	280		470	200		390	320			240	
Base Capacity (vph)	239	733	676	431	956	724	414	1245	892	410	1773
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.46	0.87	0.43	1.09	0.74	0.19	0.85	0.61	0.38	0.68	0.89

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

























Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 12: Staples Mill Road & E Parham Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	102	587	267	432	647	130	323	702	311	256	1319	131
Future Volume (veh/h)	102	587	267	432	647	130	323	702	311	256	1319	131
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1841	1870	1826	1841	1826	1870	1826	1870	1856	1856	1856
Adj Flow Rate, veh/h	111	638	0	470	703	141	351	763	0	278	1434	142
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	1	4	2	5	4	5	2	5	2	3	3	3
Cap, veh/h	175	721		436	998	594	406	1320		338	1694	168
Arrive On Green	0.05	0.21	0.00	0.13	0.29	0.29	0.12	0.38	0.00	0.10	0.36	0.36
Sat Flow, veh/h	3483	3497	1585	3374	3497	1547	3456	3469	1585	3428	4685	464
Grp Volume(v), veh/h	111	638	0	470	703	141	351	763	0	278	1034	542
Grp Sat Flow(s),veh/h/ln	1742	1749	1585	1687	1749	1547	1728	1735	1585	1714	1689	1772
Q Serve(g_s), s	3.6	20.5	0.0	15.0	20.9	7.2	11.6	20.3	0.0	9.2	32.7	32.7
Cycle Q Clear(g_c), s	3.6	20.5	0.0	15.0	20.9	7.2	11.6	20.3	0.0	9.2	32.7	32.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.26
Lane Grp Cap(c), veh/h	175	721		436	998	594	406	1320		338	1221	641
V/C Ratio(X)	0.63	0.88		1.08	0.70	0.24	0.86	0.58		0.82	0.85	0.85
Avail Cap(c_a), veh/h	240	739		436	998	594	417	1320		414	1221	641
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.0	44.7	0.0	50.5	37.1	24.2	50.3	28.5	0.0	51.3	34.1	34.1
Incr Delay (d2), s/veh	2.8	14.9	0.0	65.4	4.2	0.9	16.4	1.8	0.0	9.9	7.3	13.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.7	10.3	0.0	10.3	9.4	2.8	5.9	8.7	0.0	4.4	14.4	16.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.8	59.6	0.0	115.9	41.3	25.2	66.7	30.4	0.0	61.1	41.4	47.1
LnGrp LOS	E	E		F	D	C	E	C		E	D	D
Approach Vol, veh/h		749	A		1314			1114	A		1854	
Approach Delay, s/veh		59.2			66.2			41.8			46.0	
Approach LOS		E			E			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	16.4	50.1	10.8	38.6	18.6	48.0	20.0	29.4				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.5	5.0	6.0	5.0	5.5				
Max Green Setting (Gmax), s	14.0	41.0	8.0	31.5	14.0	41.0	15.0	24.5				
Max Q Clear Time (g_c+I1), s	11.2	22.3	5.6	22.9	13.6	34.7	17.0	22.5				
Green Ext Time (p_c), s	0.2	12.8	0.0	6.3	0.1	6.2	0.0	1.4				

Intersection Summary

HCM 6th Ctrl Delay	52.3
HCM 6th LOS	D

Notes

Unsignalized Delay for [NBR, EBR] is excluded from calculations of the approach delay and intersection delay.



Queues

13: Staples Mill Road & Hermitage Road / Commercial Ent.

10/12/2021
























Lane Group	EBT	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	12	250	160	14	1350	155	101	2085
v/c Ratio	0.12	0.98	0.30	0.14	0.46	0.16	0.58	0.59
Control Delay	49.1	101.6	9.2	55.2	14.7	2.5	62.6	11.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.1	101.6	9.2	55.2	14.7	2.5	62.6	11.6
Queue Length 50th (ft)	7	189	21	10	183	0	73	198
Queue Length 95th (ft)	28	#355	58	32	300	33	128	477
Internal Link Dist (ft)	192	548			450			520
Turn Bay Length (ft)			225	315		395	165	
Base Capacity (vph)	109	255	575	108	2964	987	224	3505
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.98	0.28	0.13	0.46	0.16	0.45	0.59

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 13: Staples Mill Road & Hermitage Road / Commercial Ent.

10/12/2021

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	8	1	2	228	2	147	13	1242	143	93	1902	17	
Future Volume (vph)	8	1	2	228	2	147	13	1242	143	93	1902	17	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0		
Lane Util. Factor		1.00			1.00	1.00	1.00	0.91	1.00	1.00	0.91		
Frt		0.98			1.00	0.85	1.00	1.00	0.85	1.00	1.00		
Flt Protected		0.96			0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (prot)		1790			1741	1583	1805	4988	1553	1736	5031		
Flt Permitted		0.96			0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (perm)		1790			1741	1583	1805	4988	1553	1736	5031		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	9	1	2	248	2	160	14	1350	155	101	2067	18	
RTOR Reduction (vph)	0	2	0	0	0	92	0	0	68	0	1	0	
Lane Group Flow (vph)	0	10	0	0	250	68	14	1350	87	101	2084	0	
Heavy Vehicles (%)	0%	0%	0%	4%	0%	2%	0%	4%	4%	4%	3%	0%	
Turn Type	Split	NA		Split	NA	pm+ov	Prot	NA	Perm	Prot	NA		
Protected Phases	4	4		3	3	1	5	2		1	6		
Permitted Phases						3			2				
Actuated Green, G (s)		1.4			17.0	28.7	2.8	64.9	64.9	11.7	73.8		
Effective Green, g (s)		1.4			17.0	28.7	2.8	64.9	64.9	11.7	73.8		
Actuated g/C Ratio		0.01			0.15	0.25	0.02	0.56	0.56	0.10	0.64		
Clearance Time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0		
Vehicle Extension (s)		3.0			3.0	3.0	3.0	5.0	5.0	3.0	5.0		
Lane Grp Cap (vph)		21			255	391	43	2790	868	175	3200		
v/s Ratio Prot		c0.01			c0.14	0.02	0.01	0.27		c0.06	c0.41		
v/s Ratio Perm						0.03			0.06				
v/c Ratio		0.48			0.98	0.17	0.33	0.48	0.10	0.58	0.65		
Uniform Delay, d1		56.9			49.3	34.3	55.7	15.4	11.9	49.8	13.1		
Progression Factor		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2		16.1			50.7	0.2	4.4	0.6	0.2	4.6	1.0		
Delay (s)		73.1			100.0	34.5	60.1	16.0	12.2	54.3	14.2		
Level of Service		E			F	C	E	B	B	D	B		
Approach Delay (s)		73.1			74.5			16.0			16.0		
Approach LOS		E			E			B			B		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			22.0		HCM 2000 Level of Service					C			
HCM 2000 Volume to Capacity ratio			0.72										
Actuated Cycle Length (s)			116.0		Sum of lost time (s)					21.0			
Intersection Capacity Utilization			71.5%		ICU Level of Service					C			
Analysis Period (min)			15										
c	Critical Lane Group												

Queues

14: Staples Mill Road & Wistar Road

10/12/2021
























Lane Group	EBT	EBR	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	117	190	66	101	1536	24	2177	114
v/c Ratio	0.67	0.52	0.36	0.56	0.44	0.21	0.70	0.11
Control Delay	66.4	11.6	32.8	60.5	9.3	55.2	17.4	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.4	11.6	32.8	60.5	9.3	55.2	17.4	1.7
Queue Length 50th (ft)	84	0	24	73	191	17	379	0
Queue Length 95th (ft)	144	64	67	125	254	45	513	20
Internal Link Dist (ft)	312		290		483		269	
Turn Bay Length (ft)		215		415		275		1000
Base Capacity (vph)	214	403	216	282	3482	163	3119	1021
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.55	0.47	0.31	0.36	0.44	0.15	0.70	0.11

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 14: Staples Mill Road & Wistar Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	95	13	175	30	3	28	93	1375	38	22	2003	105
Future Volume (vph)	95	13	175	30	3	28	93	1375	38	22	2003	105
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0		6.0		5.5	6.0		5.5	6.0	6.0
Lane Util. Factor		1.00	1.00		1.00		1.00	0.91		1.00	0.91	1.00
Frt		1.00	0.85		0.94		1.00	1.00		1.00	1.00	0.85
Flt Protected		0.96	1.00		0.98		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)		1758	1568		1649		1770	4973		1805	5036	1568
Flt Permitted		0.75	1.00		0.73		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)		1382	1568		1230		1770	4973		1805	5036	1568
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	103	14	190	33	3	30	101	1495	41	24	2177	114
RTOR Reduction (vph)	0	0	166	0	26	0	0	2	0	0	0	43
Lane Group Flow (vph)	0	117	24	0	40	0	101	1534	0	24	2177	71
Heavy Vehicles (%)	4%	0%	3%	11%	0%	0%	2%	4%	0%	0%	3%	3%
Turn Type	Perm	NA	Perm	Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4			4		5	2		1	6	
Permitted Phases	4		4	4								6
Actuated Green, G (s)		14.7	14.7		14.7		11.9	79.0		4.8	71.9	71.9
Effective Green, g (s)		14.7	14.7		14.7		11.9	79.0		4.8	71.9	71.9
Actuated g/C Ratio		0.13	0.13		0.13		0.10	0.68		0.04	0.62	0.62
Clearance Time (s)		6.0	6.0		6.0		5.5	6.0		5.5	6.0	6.0
Vehicle Extension (s)		3.5	3.5		3.5		3.0	5.0		3.0	5.0	5.0
Lane Grp Cap (vph)		175	198		155		181	3386		74	3121	971
v/s Ratio Prot							c0.06	0.31		0.01	c0.43	
v/s Ratio Perm		c0.08	0.02		0.03							0.05
v/c Ratio		0.67	0.12		0.26		0.56	0.45		0.32	0.70	0.07
Uniform Delay, d1		48.3	44.9		45.7		49.5	8.5		54.0	14.8	8.8
Progression Factor		1.00	1.00		1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2		9.7	0.3		1.0		3.7	0.4		2.5	1.3	0.1
Delay (s)		58.0	45.3		46.8		53.2	9.0		56.6	16.1	8.9
Level of Service		E	D		D		D	A		E	B	A
Approach Delay (s)		50.1			46.8			11.7			16.2	
Approach LOS		D			D			B			B	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			17.3				HCM 2000 Level of Service				B	
HCM 2000 Volume to Capacity ratio			0.68									
Actuated Cycle Length (s)			116.0				Sum of lost time (s)			17.5		
Intersection Capacity Utilization			70.4%				ICU Level of Service			C		
Analysis Period (min)			15									
c	Critical Lane Group											

Queues

15: Staples Mill Road & Bremner Boulevard

10/12/2021
























Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	51	156	77	14	69	1563	48	2317
v/c Ratio	0.39	0.59	0.67	0.07	0.49	0.49	0.35	0.73
Control Delay	58.9	17.2	80.2	0.6	61.8	13.2	57.7	18.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.9	17.2	80.2	0.6	61.8	13.2	57.7	18.9
Queue Length 50th (ft)	37	0	57	0	50	230	35	451
Queue Length 95th (ft)	75	63	#126	0	95	310	73	601
Internal Link Dist (ft)	178		254			316		213
Turn Bay Length (ft)		115		100	315		320	
Base Capacity (vph)	179	306	119	213	209	3201	171	3182
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.51	0.65	0.07	0.33	0.49	0.28	0.73

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 15: Staples Mill Road & Bremner Boulevard

10/12/2021

														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations														
Traffic Volume (vph)	38	10	147	68	5	13	65	1443	26	45	2119	59		
Future Volume (vph)	38	10	147	68	5	13	65	1443	26	45	2119	59		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Total Lost time (s)		5.0	5.0		5.0	5.0		6.0		5.0	6.0			
Lane Util. Factor		1.00	1.00		1.00	1.00		0.91		1.00	0.91			
Frt		1.00	0.85		1.00	0.85		1.00		1.00	1.00			
Flt Protected		0.96	1.00		0.96	1.00		0.95		0.95	1.00			
Satd. Flow (prot)		1733	1615		1734	1455		1736		1805	5019			
Flt Permitted		0.96	1.00		0.96	1.00		0.95		0.95	1.00			
Satd. Flow (perm)		1733	1615		1734	1455		1736		1805	5019			
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94		
Adj. Flow (vph)	40	11	156	72	5	14	69	1535	28	48	2254	63		
RTOR Reduction (vph)	0	0	144	0	0	13	0	1	0	0	2	0		
Lane Group Flow (vph)	0	51	12	0	77	1	69	1562	0	48	2315	0		
Heavy Vehicles (%)	7%	0%	0%	5%	0%	11%	4%	4%	0%	0%	3%	0%		
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA		Prot	NA			
Protected Phases	4	4		3	3		5	2		1	6			
Permitted Phases			4			3								
Actuated Green, G (s)		8.8	8.8		6.4	6.4	8.3	72.6		7.2	71.5			
Effective Green, g (s)		8.8	8.8		6.4	6.4	8.3	72.6		7.2	71.5			
Actuated g/C Ratio		0.08	0.08		0.06	0.06	0.07	0.63		0.06	0.62			
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0			
Vehicle Extension (s)		3.0	3.0		3.0	3.0	2.5	6.0		2.5	6.0			
Lane Grp Cap (vph)		131	122		95	80	124	3115		112	3093			
v/s Ratio Prot		c0.03			c0.04		c0.04	0.31		0.03	c0.46			
v/s Ratio Perm			0.01			0.00								
v/c Ratio		0.39	0.10		0.81	0.01	0.56	0.50		0.43	0.75			
Uniform Delay, d1		51.0	49.9		54.2	51.8	52.1	11.8		52.4	15.8			
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00			
Incremental Delay, d2		1.9	0.3		38.8	0.0	4.3	0.6		1.9	1.7			
Delay (s)		53.0	50.2		93.0	51.9	56.3	12.4		54.3	17.6			
Level of Service		D	D		F	D	E	B		D	B			
Approach Delay (s)		50.9			86.7			14.3			18.3			
Approach LOS		D			F			B			B			
<b>Intersection Summary</b>														
HCM 2000 Control Delay			19.8									HCM 2000 Level of Service	B	
HCM 2000 Volume to Capacity ratio			0.70											
Actuated Cycle Length (s)			116.0								21.0			
Intersection Capacity Utilization			70.5%										ICU Level of Service	C
Analysis Period (min)			15											
c	Critical Lane Group													

Queues

16: Staples Mill Road & Amtrak Station

10/12/2021



Lane Group	WBL	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	16	7	1663	3	2489
v/c Ratio	0.14	0.06	0.38	0.04	0.55
Control Delay	53.6	29.4	3.3	52.7	2.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	53.6	29.4	3.3	52.7	2.9
Queue Length 50th (ft)	12	0	83	2	166
Queue Length 95th (ft)	35	15	212	12	214
Internal Link Dist (ft)	469		187		179
Turn Bay Length (ft)				170	
Base Capacity (vph)	182	169	4342	116	4514
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.09	0.04	0.38	0.03	0.55
Intersection Summary					

# HCM Signalized Intersection Capacity Analysis

## 16: Staples Mill Road & Amtrak Station

10/12/2021



Movement	WBL	WBR	NBU	NBT	NBR	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	15	7	0	1542	22	3	2340
Future Volume (vph)	15	7	0	1542	22	3	2340
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3		5.9		6.8	5.9
Lane Util. Factor	1.00	1.00		0.91		1.00	0.91
Frt	1.00	0.85		1.00		1.00	1.00
Flt Protected	0.95	1.00		1.00		0.95	1.00
Satd. Flow (prot)	1805	1615		4975		1203	5036
Flt Permitted	0.95	1.00		1.00		0.95	1.00
Satd. Flow (perm)	1805	1615		4975		1203	5036
Peak-hour factor, PHF	0.94	0.94	0.92	0.94	0.94	0.94	0.94
Adj. Flow (vph)	16	7	0	1640	23	3	2489
RTOR Reduction (vph)	0	7	0	1	0	0	0
Lane Group Flow (vph)	16	0	0	1662	0	3	2489
Heavy Vehicles (%)	0%	0%	2%	4%	7%	50%	3%
Turn Type	Prot	Perm	Prot	NA		Prot	NA
Protected Phases	4		5	2		1	6
Permitted Phases		4					
Actuated Green, G (s)	4.7	4.7		90.9		1.4	99.1
Effective Green, g (s)	4.7	4.7		90.9		1.4	99.1
Actuated g/C Ratio	0.04	0.04		0.78		0.01	0.85
Clearance Time (s)	6.3	6.3		5.9		6.8	5.9
Vehicle Extension (s)	3.5	3.5		5.5		3.0	5.5
Lane Grp Cap (vph)	73	65		3898		14	4302
v/s Ratio Prot	c0.01			0.33		0.00	c0.49
v/s Ratio Perm		0.00					
v/c Ratio	0.22	0.00		0.43		0.21	0.58
Uniform Delay, d1	53.9	53.4		4.1		56.8	2.4
Progression Factor	1.00	1.00		1.00		1.00	1.00
Incremental Delay, d2	1.8	0.0		0.3		7.6	0.6
Delay (s)	55.7	53.4		4.4		64.3	3.0
Level of Service	E	D		A		E	A
Approach Delay (s)	55.0			4.4			3.1
Approach LOS	D			A			A

### Intersection Summary

HCM 2000 Control Delay	3.9	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.60		
Actuated Cycle Length (s)	116.0	Sum of lost time (s)	19.0
Intersection Capacity Utilization	61.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			



Queues

17: Staples Mill Road & Crockett Street

10/12/2021

























Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	53	41	27	4	49	1551	14	27	2351	52
v/c Ratio	0.42	0.18	0.25	0.02	0.40	0.43	0.01	0.26	0.67	0.05
Control Delay	60.6	1.7	57.5	0.2	62.0	5.5	0.0	57.5	15.5	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.6	1.7	57.5	0.2	62.0	5.5	0.0	57.5	15.5	0.1
Queue Length 50th (ft)	39	0	20	0	37	110	0	20	427	0
Queue Length 95th (ft)	79	0	50	0	m51	m117	m0	50	601	0
Internal Link Dist (ft)	284		261			193			145	
Turn Bay Length (ft)		115			190		75	295		105
Base Capacity (vph)	182	276	156	216	167	3637	1208	186	3520	1139
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.15	0.17	0.02	0.29	0.43	0.01	0.15	0.67	0.05

Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

HCM Signalized Intersection Capacity Analysis  
 17: Staples Mill Road & Crockett Street

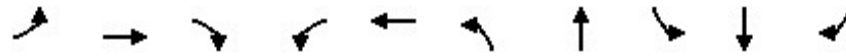
10/12/2021

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	49	2	40	23	3	4	48	1504	14	26	2280	50	
Future Volume (vph)	49	2	40	23	3	4	48	1504	14	26	2280	50	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0	
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	0.91	1.00	1.00	0.91	1.00	
Frbp, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.98	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected		0.95	1.00		0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)		1762	1615		1819	1214	1752	4988	1615	1805	5036	1581	
Flt Permitted		0.95	1.00		0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (perm)		1762	1615		1819	1214	1752	4988	1615	1805	5036	1581	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	51	2	41	24	3	4	49	1551	14	27	2351	52	
RTOR Reduction (vph)	0	0	38	0	0	4	0	0	5	0	0	18	
Lane Group Flow (vph)	0	53	3	0	27	0	49	1551	9	27	2351	34	
Confl. Peds. (#/hr)							1					1	
Heavy Vehicles (%)	3%	0%	0%	0%	0%	33%	3%	4%	0%	0%	3%	0%	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	
Protected Phases	3	3		4	4		5	2		1	6		
Permitted Phases			3			4			2			6	
Actuated Green, G (s)		7.4	7.4		4.6	4.6	7.1	78.4	78.4	4.6	75.9	75.9	
Effective Green, g (s)		7.4	7.4		4.6	4.6	7.1	78.4	78.4	4.6	75.9	75.9	
Actuated g/C Ratio		0.06	0.06		0.04	0.04	0.06	0.68	0.68	0.04	0.65	0.65	
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0	
Vehicle Extension (s)		2.5	2.5		2.5	2.5	2.5	4.0	4.0	2.5	4.0	4.0	
Lane Grp Cap (vph)		112	103		72	48	107	3371	1091	71	3295	1034	
v/s Ratio Prot		c0.03			c0.01		c0.03	0.31		0.01	c0.47		
v/s Ratio Perm			0.00			0.00			0.01			0.02	
v/c Ratio		0.47	0.03		0.38	0.00	0.46	0.46	0.01	0.38	0.71	0.03	
Uniform Delay, d1		52.4	50.9		54.3	53.5	52.6	8.8	6.1	54.3	13.0	7.1	
Progression Factor		1.00	1.00		1.00	1.00	1.11	0.55	1.00	1.00	1.00	1.00	
Incremental Delay, d2		2.3	0.1		2.4	0.0	1.3	0.3	0.0	2.5	1.3	0.1	
Delay (s)		54.7	51.0		56.7	53.5	59.9	5.1	6.1	56.8	14.3	7.1	
Level of Service		D	D		E	D	E	A	A	E	B	A	
Approach Delay (s)		53.1			56.3			6.8			14.7		
Approach LOS		D			E			A			B		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			12.8		HCM 2000 Level of Service					B			
HCM 2000 Volume to Capacity ratio			0.66										
Actuated Cycle Length (s)			116.0		Sum of lost time (s)					21.0			
Intersection Capacity Utilization			65.7%		ICU Level of Service					C			
Analysis Period (min)			15										
c Critical Lane Group													

Queues

18: Staples Mill Road & Hilliard Road / Glenside Drive

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	152	378	151	254	873	121	1429	214	2025	296
v/c Ratio	0.81	0.79	0.44	0.93	1.32	0.67	0.71	0.82	0.90	0.36
Control Delay	81.2	61.8	11.6	87.9	190.9	68.4	32.0	73.9	35.7	11.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	81.2	61.8	11.6	87.9	190.9	68.4	32.0	73.9	35.7	11.3
Queue Length 50th (ft)	112	146	0	189	~441	87	327	171	273	36
Queue Length 95th (ft)	#215	#218	61	#342	#572	152	384	#278	#432	143
Internal Link Dist (ft)		762			867		276		418	
Turn Bay Length (ft)	200		215	725		280		330		245
Base Capacity (vph)	202	479	345	280	661	202	2001	280	2253	832
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.75	0.79	0.44	0.91	1.32	0.60	0.71	0.76	0.90	0.36

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

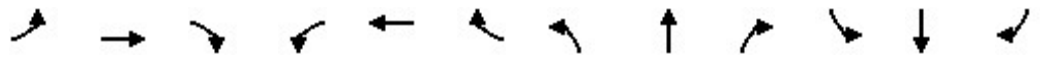
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 18: Staples Mill Road & Hilliard Road / Glenside Drive

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑		↘	↑↑↑		↘	↑↑↑	↗
Traffic Volume (veh/h)	144	359	143	241	582	247	115	1194	163	203	1924	281
Future Volume (veh/h)	144	359	143	241	582	247	115	1194	163	203	1924	281
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	152	378	151	254	613	260	121	1257	172	214	2025	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	180	444	198	281	441	187	148	1875	257	243	2380	
Arrive On Green	0.10	0.12	0.12	0.16	0.18	0.18	0.08	0.41	0.41	0.13	0.46	0.00
Sat Flow, veh/h	1810	3610	1610	1810	2470	1047	1810	4613	631	1810	5187	1610
Grp Volume(v), veh/h	152	378	151	254	448	425	121	942	487	214	2025	0
Grp Sat Flow(s),veh/h/ln	1810	1805	1610	1810	1805	1712	1810	1729	1786	1810	1729	1610
Q Serve(g_s), s	9.6	11.9	10.5	16.0	20.7	20.7	7.6	25.8	25.8	13.5	40.2	0.0
Cycle Q Clear(g_c), s	9.6	11.9	10.5	16.0	20.7	20.7	7.6	25.8	25.8	13.5	40.2	0.0
Prop In Lane	1.00		1.00	1.00		0.61	1.00		0.35	1.00		1.00
Lane Grp Cap(c), veh/h	180	444	198	281	323	306	148	1406	726	243	2380	
V/C Ratio(X)	0.85	0.85	0.76	0.90	1.39	1.39	0.82	0.67	0.67	0.88	0.85	
Avail Cap(c_a), veh/h	203	467	208	281	323	306	203	1406	726	281	2380	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	51.4	49.8	49.2	48.2	47.6	47.6	52.4	28.1	28.1	49.3	27.9	0.0
Incr Delay (d2), s/veh	22.4	13.6	14.7	29.7	192.9	194.4	16.4	2.6	4.9	23.6	4.1	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.5	6.2	5.0	9.5	26.5	25.3	4.1	11.0	11.9	7.7	17.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	73.7	63.5	63.9	77.8	240.5	242.0	68.8	30.6	33.0	72.9	31.9	0.0
LnGrp LOS	E	E	E	E	F	F	E	C	C	E	C	
Approach Vol, veh/h		681			1127			1550			2239	A
Approach Delay, s/veh		65.8			204.4			34.4			35.8	
Approach LOS		E			F			C			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	20.6	53.1	23.0	19.3	14.5	59.2	16.5	25.7				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	18.0	44.0	18.0	15.0	13.0	49.0	13.0	20.0				
Max Q Clear Time (g_c+I1), s	15.5	27.8	18.0	13.9	9.6	42.2	11.6	22.7				
Green Ext Time (p_c), s	0.1	11.1	0.0	0.4	0.1	6.3	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay	73.0
HCM 6th LOS	E

Notes

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

Queues

19: Staples Mill Road & Aspen Avenue / Townhouse Road

10/12/2021



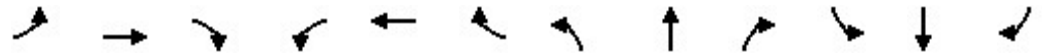
Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	136	86	37	1593	49	2304
v/c Ratio	0.76	0.59	0.24	0.48	0.21	0.67
Control Delay	60.6	62.0	9.3	12.5	7.6	14.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.6	62.0	9.3	12.5	7.6	14.8
Queue Length 50th (ft)	66	55	8	238	10	429
Queue Length 95th (ft)	#159	109	18	284	22	499
Internal Link Dist (ft)	218	764		385		854
Turn Bay Length (ft)			230		215	
Base Capacity (vph)	190	162	175	3342	252	3457
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.72	0.53	0.21	0.48	0.19	0.67

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 19: Staples Mill Road & Aspen Avenue / Townhouse Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↑↑↑		↖	↑↑↑	
Traffic Volume (vph)	45	10	74	59	5	18	35	1452	62	47	2168	21
Future Volume (vph)	45	10	74	59	5	18	35	1452	62	47	2168	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0		5.0	6.0		5.0	6.0	
Lane Util. Factor		1.00			1.00		1.00	0.91		1.00	0.91	
Frt		0.92			0.97		1.00	0.99		1.00	1.00	
Flt Protected		0.98			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1723			1779		1805	5155		1805	5180	
Flt Permitted		0.98			0.97		0.05	1.00		0.12	1.00	
Satd. Flow (perm)		1723			1779		104	5155		220	5180	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	47	11	78	62	5	19	37	1528	65	49	2282	22
RTOR Reduction (vph)	0	42	0	0	9	0	0	3	0	0	1	0
Lane Group Flow (vph)	0	94	0	0	77	0	37	1590	0	49	2303	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Split	NA		Split	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases							2			6		
Actuated Green, G (s)		9.2			7.7		76.9	73.2		79.3	74.4	
Effective Green, g (s)		9.2			7.7		76.9	73.2		79.3	74.4	
Actuated g/C Ratio		0.08			0.07		0.66	0.63		0.68	0.64	
Clearance Time (s)		5.0			5.0		5.0	6.0		5.0	6.0	
Vehicle Extension (s)		3.0			3.0		2.5	4.0		2.5	4.0	
Lane Grp Cap (vph)		136			118		123	3252		217	3322	
v/s Ratio Prot		c0.05			c0.04		c0.01	0.31		0.01	c0.44	
v/s Ratio Perm							0.19			0.14		
v/c Ratio		0.69			0.65		0.30	0.49		0.23	0.69	
Uniform Delay, d1		52.0			52.8		11.5	11.4		7.3	13.4	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		13.6			11.7		1.0	0.5		0.4	1.2	
Delay (s)		65.6			64.5		12.5	11.9		7.6	14.6	
Level of Service		E			E		B	B		A	B	
Approach Delay (s)		65.6			64.5		12.0			14.5		
Approach LOS		E			E		B			B		
<b>Intersection Summary</b>												
HCM 2000 Control Delay			16.2				HCM 2000 Level of Service				B	
HCM 2000 Volume to Capacity ratio			0.67									
Actuated Cycle Length (s)			116.0				Sum of lost time (s)			21.0		
Intersection Capacity Utilization			60.4%				ICU Level of Service				B	
Analysis Period (min)			15									
c Critical Lane Group												

Queues

20: Staples Mill Road & Dumbarton Road / Wharfside Road

10/12/2021



Lane Group	EBL	EBT	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	8	17	1018	269	1358	390	299	2090
v/c Ratio	0.09	0.18	0.99	0.29	0.76	0.48	1.01	0.73
Control Delay	53.5	47.6	68.4	5.5	37.1	5.4	104.5	21.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	53.5	47.6	68.4	5.5	37.1	5.4	104.5	21.4
Queue Length 50th (ft)	6	10	~451	22	327	5	~230	409
Queue Length 95th (ft)	22	34	#603	73	384	73	#410	468
Internal Link Dist (ft)		212			241			321
Turn Bay Length (ft)				250		230	235	
Base Capacity (vph)	171	169	1025	914	1788	805	295	2861
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.10	0.99	0.29	0.76	0.48	1.01	0.73

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 20: Staples Mill Road & Dumbarton Road / Wharfside Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↗		↘↗		↗		↑↑↑	↗	↘	↑↑↑	
Traffic Volume (vph)	8	8	9	977	0	258	0	1304	374	287	2006	0
Future Volume (vph)	8	8	9	977	0	258	0	1304	374	287	2006	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0		5.0		6.0	6.0	5.0	6.0	
Lane Util. Factor	1.00	1.00		0.97		1.00		0.91	1.00	1.00	0.91	
Frt	1.00	0.92		1.00		0.85		1.00	0.85	1.00	1.00	
Flt Protected	0.95	1.00		0.95		1.00		1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1805	1749		3502		1615		5187	1615	1805	5187	
Flt Permitted	0.95	1.00		0.95		1.00		1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1805	1749		3502		1615		5187	1615	1805	5187	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	8	8	9	1018	0	269	0	1358	390	299	2090	0
RTOR Reduction (vph)	0	4	0	0	0	117	0	0	255	0	0	0
Lane Group Flow (vph)	8	13	0	1018	0	152	0	1358	135	299	2090	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Split	NA		Prot		pm+ov		NA	Perm	Prot	NA	
Protected Phases	3	3		4		1		2		1	6	
Permitted Phases						4			2			
Actuated Green, G (s)	4.0	4.0		34.0		53.0		38.0	38.0	19.0	62.0	
Effective Green, g (s)	4.0	4.0		34.0		53.0		38.0	38.0	19.0	62.0	
Actuated g/C Ratio	0.03	0.03		0.29		0.46		0.33	0.33	0.16	0.53	
Clearance Time (s)	5.0	5.0		5.0		5.0		6.0	6.0	5.0	6.0	
Vehicle Extension (s)	2.5	2.5		2.5		2.5		6.0	6.0	2.5	6.0	
Lane Grp Cap (vph)	62	60		1026		807		1699	529	295	2772	
v/s Ratio Prot	0.00	c0.01		c0.29		0.03		0.26		c0.17	c0.40	
v/s Ratio Perm						0.06			0.08			
v/c Ratio	0.13	0.22		0.99		0.19		0.80	0.26	1.01	0.75	
Uniform Delay, d1	54.3	54.5		40.9		18.7		35.5	28.6	48.5	21.1	
Progression Factor	1.00	1.00		1.00		1.00		1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.7	1.3		26.1		0.1		4.0	1.2	55.9	2.0	
Delay (s)	55.0	55.8		66.9		18.8		39.6	29.8	104.4	23.0	
Level of Service	D	E		E		B		D	C	F	C	
Approach Delay (s)		55.6			56.9			37.4			33.2	
Approach LOS		E			E			D			C	

### Intersection Summary

HCM 2000 Control Delay	40.2	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.89		
Actuated Cycle Length (s)	116.0	Sum of lost time (s)	21.0
Intersection Capacity Utilization	88.1%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			



Queues

21: Staples Mill Road & Dickens Road / Ent. to Comcast

10/12/2021



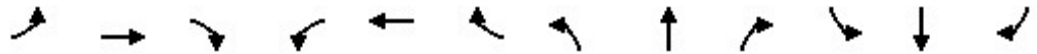
Lane Group	EBL	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	177	175	197	26	9	17	1459	29	3157
v/c Ratio	0.73	0.71	0.49	0.25	0.04	0.18	0.45	0.27	0.94
Control Delay	64.1	63.0	10.3	57.2	0.4	56.1	14.4	57.7	28.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.1	63.0	10.3	57.2	0.4	56.1	14.4	57.7	28.2
Queue Length 50th (ft)	133	131	0	19	0	12	237	21	716
Queue Length 95th (ft)	209	206	64	48	0	37	320	51	#1153
Internal Link Dist (ft)		248		223			466		240
Turn Bay Length (ft)			105		50	450		180	
Base Capacity (vph)	279	280	427	174	263	124	3251	202	3345
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.63	0.63	0.46	0.15	0.03	0.14	0.45	0.14	0.94

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 21: Staples Mill Road & Dickens Road / Ent. to Comcast

10/12/2021

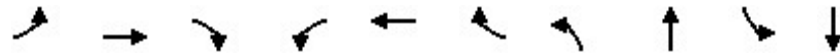


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↖	↗	↘		↖	↗	↖	↑↑↑		↖	↑↑↑		
Traffic Volume (vph)	324	10	187	16	9	9	16	1335	51	28	2637	362	
Future Volume (vph)	324	10	187	16	9	9	16	1335	51	28	2637	362	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.0	5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0		
Lane Util. Factor	0.95	0.95	1.00		1.00	1.00	1.00	0.91		1.00	0.91		
Frt	1.00	1.00	0.85		1.00	0.85	1.00	0.99		1.00	0.98		
Flt Protected	0.95	0.96	1.00		0.97	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (prot)	1715	1724	1615		1840	1615	1805	5158		1805	5093		
Flt Permitted	0.95	0.96	1.00		0.97	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (perm)	1715	1724	1615		1840	1615	1805	5158		1805	5093		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Adj. Flow (vph)	341	11	197	17	9	9	17	1405	54	29	2776	381	
RTOR Reduction (vph)	0	0	169	0	0	9	0	2	0	0	12	0	
Lane Group Flow (vph)	177	175	28	0	26	0	17	1457	0	29	3145	0	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA		Prot	NA		
Protected Phases	4	4		3	3		5	2		1	6		
Permitted Phases			4			3							
Actuated Green, G (s)	16.6	16.6	16.6		4.6	4.6	2.9	69.0		4.8	70.9		
Effective Green, g (s)	16.6	16.6	16.6		4.6	4.6	2.9	69.0		4.8	70.9		
Actuated g/C Ratio	0.14	0.14	0.14		0.04	0.04	0.02	0.59		0.04	0.61		
Clearance Time (s)	5.0	5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0		
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5	2.5	6.0		2.5	6.0		
Lane Grp Cap (vph)	245	246	231		72	64	45	3068		74	3112		
v/s Ratio Prot	c0.10	0.10			c0.01		0.01	0.28		c0.02	c0.62		
v/s Ratio Perm			0.02			0.00							
v/c Ratio	0.72	0.71	0.12		0.36	0.01	0.38	0.47		0.39	1.01		
Uniform Delay, d1	47.5	47.4	43.3		54.3	53.5	55.7	13.3		54.2	22.5		
Progression Factor	1.00	1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00		
Incremental Delay, d2	9.5	8.7	0.2		2.2	0.0	3.8	0.5		2.5	18.8		
Delay (s)	57.0	56.1	43.5		56.5	53.5	59.5	13.8		56.7	41.3		
Level of Service	E	E	D		E	D	E	B		E	D		
Approach Delay (s)		51.9			55.7			14.3			41.5		
Approach LOS		D			E			B			D		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			35.0		HCM 2000 Level of Service						D		
HCM 2000 Volume to Capacity ratio			0.92										
Actuated Cycle Length (s)			116.0		Sum of lost time (s)					21.0			
Intersection Capacity Utilization			88.1%		ICU Level of Service					E			
Analysis Period (min)			15										
c Critical Lane Group													

Queues

22: Brook Road & E Parham Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	172	705	93	142	813	236	122	584	371	705
v/c Ratio	0.60	0.48	0.14	0.54	0.80	0.33	0.49	0.46	0.83	0.46
Control Delay	60.0	34.8	3.6	59.5	45.4	11.4	58.9	35.5	66.0	31.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.0	34.8	3.6	59.5	45.4	11.4	58.9	35.5	66.0	31.4
Queue Length 50th (ft)	65	154	0	54	290	52	46	131	140	146
Queue Length 95th (ft)	100	199	26	86	376	110	76	169	#214	194
Internal Link Dist (ft)		437			433			569		731
Turn Bay Length (ft)	415		350	300			400		550	
Base Capacity (vph)	370	1503	711	418	1052	714	398	1268	466	1546
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.46	0.47	0.13	0.34	0.77	0.33	0.31	0.46	0.80	0.46

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM 6th Signalized Intersection Summary

## 22: Brook Road & E Parham Road

10/12/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	158	649	86	131	748	217	112	419	119	341	481	167
Future Volume (veh/h)	158	649	86	131	748	217	112	419	119	341	481	167
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1781	1826	1633	1752	1856	1826	1781	1856	1856	1885	1856	1856
Adj Flow Rate, veh/h	172	705	93	142	813	236	122	455	129	371	523	182
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	8	5	18	10	3	5	8	3	3	1	3	3
Cap, veh/h	228	1409	471	196	967	613	192	1187	325	427	1364	460
Arrive On Green	0.07	0.28	0.28	0.06	0.27	0.27	0.06	0.30	0.30	0.12	0.36	0.36
Sat Flow, veh/h	3291	4985	1383	3237	3526	1546	3291	3952	1084	3483	3741	1263
Grp Volume(v), veh/h	172	705	93	142	813	236	122	387	197	371	471	234
Grp Sat Flow(s),veh/h/ln	1646	1662	1383	1618	1763	1546	1646	1689	1659	1742	1689	1627
Q Serve(g_s), s	6.1	13.9	5.6	5.1	25.7	12.8	4.3	10.7	11.2	12.3	12.1	12.6
Cycle Q Clear(g_c), s	6.1	13.9	5.6	5.1	25.7	12.8	4.3	10.7	11.2	12.3	12.1	12.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.65	1.00		0.78
Lane Grp Cap(c), veh/h	228	1409	471	196	967	613	192	1014	498	427	1231	593
V/C Ratio(X)	0.76	0.50	0.20	0.72	0.84	0.38	0.64	0.38	0.40	0.87	0.38	0.40
Avail Cap(c_a), veh/h	377	1409	471	425	1037	644	404	1014	498	458	1231	593
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.9	35.4	27.5	54.4	40.4	25.3	54.3	32.6	32.8	50.8	27.7	27.8
Incr Delay (d2), s/veh	1.9	0.5	0.3	1.9	6.6	0.7	1.3	1.1	2.4	14.7	0.9	2.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.6	5.7	1.9	2.1	11.9	4.8	1.8	4.5	4.8	6.2	5.1	5.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	55.9	35.8	27.8	56.3	47.0	26.0	55.7	33.7	35.1	65.5	28.6	29.8
LnGrp LOS	E	D	C	E	D	C	E	C	D	E	C	C
Approach Vol, veh/h		970			1191			706			1076	
Approach Delay, s/veh		38.6			43.9			37.9			41.6	
Approach LOS		D			D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	21.0	42.7	13.7	40.7	13.4	50.3	14.7	39.7				
Change Period (Y+Rc), s	6.5	* 7.3	6.5	* 7.3	6.5	* 7.3	6.5	7.3				
Max Green Setting (Gmax), s	15.5	* 27	15.5	* 33	14.5	* 28	13.5	34.7				
Max Q Clear Time (g_c+I1), s	14.3	13.2	7.1	15.9	6.3	14.6	8.1	27.7				
Green Ext Time (p_c), s	0.1	6.2	0.1	7.3	0.1	5.6	0.1	4.7				

### Intersection Summary

HCM 6th Ctrl Delay	40.9
HCM 6th LOS	D

### Notes

\* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Queues

23: Brook Road & Hilliard Road / Hilliard Avenue

10/12/2021



Lane Group	EBL	EBR	WBL	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	197	183	126	28	321	613	550	150
v/c Ratio	0.54	0.27	0.64	0.10	0.76	0.28	0.50	0.19
Control Delay	54.6	3.4	65.4	0.7	55.4	10.6	32.6	2.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	54.6	3.4	65.4	0.7	55.4	10.6	32.6	2.5
Queue Length 50th (ft)	74	0	92	0	219	103	182	0
Queue Length 95th (ft)	109	34	158	0	#466	147	213	27
Internal Link Dist (ft)						335	653	
Turn Bay Length (ft)		275		50	210			115
Base Capacity (vph)	472	724	215	292	424	2168	1238	817
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.25	0.59	0.10	0.76	0.28	0.44	0.18

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 23: Brook Road & Hilliard Road / Hilliard Avenue

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↖↗		↖	↖		↖	↖	↕↔			↖↗	↖	
Traffic Volume (vph)	181	0	168	116	0	26	295	547	17	10	496	138	
Future Volume (vph)	181	0	168	116	0	26	295	547	17	10	496	138	
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.5		5.5	5.5		5.5	5.5	6.0			6.0	5.5	
Lane Util. Factor	0.97		1.00	1.00		1.00	1.00	0.95			0.95	1.00	
Frpb, ped/bikes	1.00		1.00	1.00		1.00	1.00	1.00			1.00	1.00	
Flpb, ped/bikes	1.00		1.00	1.00		1.00	1.00	1.00			1.00	1.00	
Frt	1.00		0.85	1.00		0.85	1.00	1.00			1.00	0.85	
Flt Protected	0.95		1.00	0.95		1.00	0.95	1.00			1.00	1.00	
Satd. Flow (prot)	3183		1553	1805		1495	1719	3416			3459	1392	
Flt Permitted	0.95		1.00	0.95		1.00	0.95	1.00			0.94	1.00	
Satd. Flow (perm)	3183		1553	1805		1495	1719	3416			3247	1392	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	197	0	183	126	0	28	321	595	18	11	539	150	
RTOR Reduction (vph)	0	0	117	0	0	25	0	1	0	0	0	82	
Lane Group Flow (vph)	197	0	66	126	0	3	321	612	0	0	550	68	
Confl. Peds. (#/hr)	1					1	1		1	1		1	
Heavy Vehicles (%)	10%	14%	4%	0%	8%	8%	5%	5%	10%	17%	4%	16%	
Turn Type	Prot		pt+ov	Prot		Prot	Prot	NA		Perm	NA	pt+ov	
Protected Phases	3		3 5	4		4	5	2			6	3 6	
Permitted Phases										6			
Actuated Green, G (s)	13.4		42.5	12.8		12.8	29.1	74.8			40.2	59.6	
Effective Green, g (s)	13.4		42.5	12.8		12.8	29.1	74.8			40.2	53.6	
Actuated g/C Ratio	0.11		0.36	0.11		0.11	0.25	0.63			0.34	0.45	
Clearance Time (s)	5.5			5.5		5.5	5.5	6.0			6.0		
Vehicle Extension (s)	3.5			3.5		3.5	3.0	7.0			7.0		
Lane Grp Cap (vph)	361		559	195		162	423	2165			1106	632	
v/s Ratio Prot	c0.06		0.04	c0.07		0.00	c0.19	0.18				0.05	
v/s Ratio Perm											c0.17		
v/c Ratio	0.55		0.12	0.65		0.02	0.76	0.28			0.50	0.11	
Uniform Delay, d1	49.4		25.2	50.4		47.0	41.2	9.6			30.9	18.5	
Progression Factor	1.00		1.00	1.00		1.00	1.00	1.00			1.00	1.00	
Incremental Delay, d2	1.9		0.1	7.5		0.1	7.6	0.3			1.6	0.1	
Delay (s)	51.3		25.3	57.9		47.0	48.8	10.0			32.5	18.6	
Level of Service	D		C	E		D	D	A			C	B	
Approach Delay (s)		38.8			55.9			23.3			29.5		
Approach LOS		D			E			C			C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			30.3		HCM 2000 Level of Service							C	
HCM 2000 Volume to Capacity ratio			0.60										
Actuated Cycle Length (s)			118.0		Sum of lost time (s)						22.5		
Intersection Capacity Utilization			51.4%		ICU Level of Service						A		
Analysis Period (min)			15										

c Critical Lane Group

Queues

24: Brook Road & Dumbarton Road / Azalea Avenue

10/12/2021

























Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	52	342	189	1030	585	321	76	217	303	83
v/c Ratio	0.47	0.37	0.81	0.85	0.68	0.38	0.15	0.77	0.51	0.20
Control Delay	70.3	39.1	78.4	47.2	10.2	41.5	0.6	68.2	50.8	1.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	70.3	39.1	78.4	47.2	10.2	41.5	0.6	68.2	50.8	1.1
Queue Length 50th (ft)	41	115	151	421	48	115	0	170	118	0
Queue Length 95th (ft)	85	173	222	#549	182	160	0	#286	166	0
Internal Link Dist (ft)		442		279		310			329	
Turn Bay Length (ft)	210		80					150		150
Base Capacity (vph)	111	931	342	1205	864	835	518	283	594	406
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.47	0.37	0.55	0.85	0.68	0.38	0.15	0.77	0.51	0.20

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 24: Brook Road & Dumbarton Road / Azalea Avenue

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	48	287	31	176	958	544	63	235	71	202	282	77
Future Volume (vph)	48	287	31	176	958	544	63	235	71	202	282	77
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	5.0		5.8	5.0	5.0		7.1	7.1	7.0	7.0	7.0
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00
Frbp, ped/bikes	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.99		1.00	1.00	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.99	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1597	3526		1770	3505	1553		3490	1583	1687	3539	1503
Flt Permitted	0.95	1.00		0.95	1.00	1.00		0.99	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1597	3526		1770	3505	1553		3490	1583	1687	3539	1503
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	52	309	33	189	1030	585	68	253	76	217	303	83
RTOR Reduction (vph)	0	6	0	0	0	330	0	0	59	0	0	69
Lane Group Flow (vph)	52	336	0	189	1030	255	0	321	17	217	303	14
Confl. Peds. (#/hr)							1					1
Heavy Vehicles (%)	13%	1%	0%	2%	3%	4%	0%	3%	2%	7%	2%	6%
Turn Type	Prot	NA		Prot	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	3	8		7	4		1	1		2	2	
Permitted Phases						4			1			2
Actuated Green, G (s)	7.0	34.1		16.4	43.0	43.0		28.6	28.6	21.0	21.0	21.0
Effective Green, g (s)	7.0	34.1		16.4	43.0	43.0		28.6	28.6	21.0	21.0	21.0
Actuated g/C Ratio	0.06	0.27		0.13	0.34	0.34		0.23	0.23	0.17	0.17	0.17
Clearance Time (s)	6.3	5.0		5.8	5.0	5.0		7.1	7.1	7.0	7.0	7.0
Vehicle Extension (s)	2.0	0.2		0.2	2.0	2.0		4.0	4.0	5.0	5.0	5.0
Lane Grp Cap (vph)	89	961		232	1205	534		798	362	283	594	252
v/s Ratio Prot	0.03	0.10		c0.11	c0.29			c0.09		c0.13	0.09	
v/s Ratio Perm						0.16			0.01			0.01
v/c Ratio	0.58	0.35		0.81	0.85	0.48		0.40	0.05	0.77	0.51	0.06
Uniform Delay, d1	57.6	36.5		52.8	38.1	32.2		40.9	37.6	49.7	47.3	43.7
Progression Factor	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	6.2	1.0		18.4	7.8	3.0		1.5	0.3	17.9	3.1	0.4
Delay (s)	63.7	37.5		71.2	45.9	35.2		42.4	37.8	67.5	50.4	44.1
Level of Service	E	D		E	D	D		D	D	E	D	D
Approach Delay (s)		41.0			45.1			41.6			55.7	
Approach LOS		D			D			D			E	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			46.2				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			0.73									
Actuated Cycle Length (s)			125.0				Sum of lost time (s)		25.4			
Intersection Capacity Utilization			90.5%				ICU Level of Service		E			
Analysis Period (min)			15									

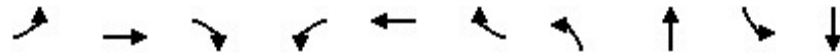
c Critical Lane Group



Queues

25: Springfield Road & Gaskins Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	138	575	72	230	853	162	75	484	134	725
v/c Ratio	0.40	0.82	0.15	0.56	1.04	0.24	0.53	0.51	0.67	0.61
Control Delay	44.4	54.5	0.7	45.3	86.1	5.1	63.6	30.7	63.9	25.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	44.4	54.5	0.7	45.3	86.1	5.1	63.6	30.7	63.9	25.8
Queue Length 50th (ft)	95	220	0	152	~374	5	54	128	96	174
Queue Length 95th (ft)	174	#367	0	236	#501	46	101	179	154	231
Internal Link Dist (ft)		455			617			1015		812
Turn Bay Length (ft)	225		135	285		165	210		450	
Base Capacity (vph)	344	703	474	413	819	751	190	988	282	1224
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.40	0.82	0.15	0.56	1.04	0.22	0.39	0.49	0.48	0.59

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.



























Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 25: Springfield Road & Gaskins Road

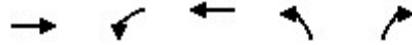
10/12/2021

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		 			 			 			 		
Traffic Volume (vph)	141	515	66	212	785	149	69	287	158	123	356	311	
Future Volume (vph)	141	515	66	212	785	149	69	287	158	123	356	311	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0		5.0	6.0		
Lane Util. Factor	0.91	0.91	1.00	1.00	0.95	1.00	1.00	0.95		1.00	0.95		
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95		1.00	0.93		
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (prot)	1643	3355	1615	1787	3539	1538	1805	3360		1787	3307		
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (perm)	1643	3355	1615	1787	3539	1538	1805	3360		1787	3307		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	153	560	72	230	853	162	75	312	172	134	387	338	
RTOR Reduction (vph)	0	0	57	0	0	98	0	66	0	0	142	0	
Lane Group Flow (vph)	138	575	15	230	853	64	75	418	0	134	583	0	
Heavy Vehicles (%)	0%	3%	0%	1%	2%	5%	0%	1%	3%	1%	2%	1%	
Turn Type	Split	NA	Perm	Split	NA	pm+ov	Prot	NA		Prot	NA		
Protected Phases	3	3		4	4		5	2		1	6		
Permitted Phases			3			4							
Actuated Green, G (s)	23.9	23.9	23.9	26.4	26.4	39.3	7.9	29.8		12.9	34.8		
Effective Green, g (s)	23.9	23.9	23.9	26.4	26.4	39.3	7.9	29.8		12.9	34.8		
Actuated g/C Ratio	0.21	0.21	0.21	0.23	0.23	0.34	0.07	0.26		0.11	0.31		
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0		5.0	6.0		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	2.0	2.0	6.0		2.0	6.0		
Lane Grp Cap (vph)	344	703	338	413	819	597	125	878		202	1009		
v/s Ratio Prot	0.08	c0.17		0.13	c0.24	0.01	0.04	0.12		c0.07	c0.18		
v/s Ratio Perm			0.01			0.03							
v/c Ratio	0.40	0.82	0.04	0.56	1.04	0.11	0.60	0.48		0.66	0.58		
Uniform Delay, d1	38.9	43.0	35.9	38.6	43.8	25.4	51.5	35.5		48.5	33.4		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00		
Incremental Delay, d2	0.8	7.3	0.1	1.6	42.8	0.0	5.1	1.8		6.2	2.4		
Delay (s)	39.6	50.3	36.0	40.3	86.6	25.4	56.6	37.4		54.7	35.8		
Level of Service	D	D	D	D	F	C	E	D		D	D		
Approach Delay (s)		47.1			70.1			39.9			38.7		
Approach LOS		D			E			D			D		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			52.2									HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio			0.80										
Actuated Cycle Length (s)			114.0									Sum of lost time (s)	21.0
Intersection Capacity Utilization			77.4%									ICU Level of Service	D
Analysis Period (min)			15										
c Critical Lane Group													

# Queues

## 26: West End Drive & Hungary Road

10/12/2021



Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1070	233	937	186	110
v/c Ratio	0.75	0.63	0.43	0.56	0.28
Control Delay	19.2	17.7	6.7	28.7	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	19.2	17.7	6.7	28.7	7.0
Queue Length 50th (ft)	157	30	75	62	0
Queue Length 95th (ft)	255	#124	134	116	33
Internal Link Dist (ft)	202		219	505	
Turn Bay Length (ft)				90	
Base Capacity (vph)	1423	368	2204	520	547
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.75	0.63	0.43	0.36	0.20

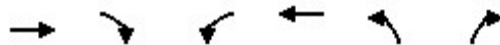
### Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 26: West End Drive & Hungary Road

10/12/2021



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	↵
Traffic Volume (vph)	697	287	214	862	171	101
Future Volume (vph)	697	287	214	862	171	101
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.5		5.5	5.5	5.0	5.0
Lane Util. Factor	0.95		1.00	0.95	1.00	1.00
Frt	0.96		1.00	1.00	1.00	0.85
Flt Protected	1.00		0.95	1.00	0.95	1.00
Satd. Flow (prot)	3394		1787	3471	1752	1583
Flt Permitted	1.00		0.13	1.00	0.95	1.00
Satd. Flow (perm)	3394		248	3471	1752	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	758	312	233	937	186	110
RTOR Reduction (vph)	41	0	0	0	0	89
Lane Group Flow (vph)	1029	0	233	937	186	21
Heavy Vehicles (%)	2%	1%	1%	4%	3%	2%
Turn Type	NA		pm+pt	NA	Prot	Perm
Protected Phases	2		1	6	4	
Permitted Phases			6			4
Actuated Green, G (s)	24.8		38.6	38.6	11.6	11.6
Effective Green, g (s)	24.8		38.6	38.6	11.6	11.6
Actuated g/C Ratio	0.41		0.64	0.64	0.19	0.19
Clearance Time (s)	5.5		5.5	5.5	5.0	5.0
Vehicle Extension (s)	6.0		3.0	6.0	3.0	3.0
Lane Grp Cap (vph)	1386		368	2207	334	302
v/s Ratio Prot	c0.30		c0.09	0.27	c0.11	
v/s Ratio Perm			0.32			0.01
v/c Ratio	0.74		0.63	0.42	0.56	0.07
Uniform Delay, d1	15.2		8.5	5.5	22.2	20.1
Progression Factor	1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2	3.0		3.5	0.4	2.0	0.1
Delay (s)	18.2		12.0	5.9	24.2	20.2
Level of Service	B		B	A	C	C
Approach Delay (s)	18.2			7.1	22.7	
Approach LOS	B			A	C	

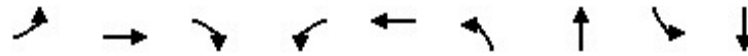
### Intersection Summary

HCM 2000 Control Delay	13.6	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.73		
Actuated Cycle Length (s)	60.7	Sum of lost time (s)	19.0
Intersection Capacity Utilization	63.1%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

Queues

27: Woodman Road & Hungary Road

10/12/2021



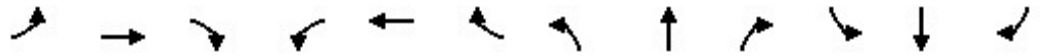
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	197	178	217	137	290	110	493	60	841
v/c Ratio	0.56	0.34	0.36	0.30	0.68	0.62	0.41	0.37	0.80
Control Delay	20.2	21.6	5.2	14.1	27.2	46.8	17.1	34.6	26.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	20.2	21.6	5.2	14.1	27.2	46.8	17.1	34.6	26.7
Queue Length 50th (ft)	48	56	0	32	86	40	73	22	144
Queue Length 95th (ft)	88	105	44	63	158	#114	123	57	#258
Internal Link Dist (ft)		244			266		277		252
Turn Bay Length (ft)	240			75		260		200	
Base Capacity (vph)	349	591	655	461	572	181	1263	170	1102
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.56	0.30	0.33	0.30	0.51	0.61	0.39	0.35	0.76

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 27: Woodman Road & Hungary Road

10/12/2021

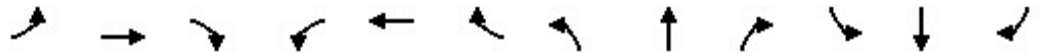


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	181	164	200	126	187	80	101	406	48	55	635	139
Future Volume (veh/h)	181	164	200	126	187	80	101	406	48	55	635	139
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1870	1885	1870	1811	1811	1826	1870	1870	1856	1856	1856
Adj Flow Rate, veh/h	197	178	217	137	203	87	110	441	52	60	690	151
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	3	2	1	2	6	6	5	2	2	3	3	3
Cap, veh/h	358	423	361	421	253	109	140	1072	126	74	853	186
Arrive On Green	0.10	0.23	0.23	0.08	0.21	0.21	0.08	0.33	0.33	0.04	0.30	0.30
Sat Flow, veh/h	1767	1870	1598	1781	1203	515	1739	3204	376	1767	2876	629
Grp Volume(v), veh/h	197	178	217	137	0	290	110	244	249	60	423	418
Grp Sat Flow(s),veh/h/ln	1767	1870	1598	1781	0	1718	1739	1777	1803	1767	1763	1742
Q Serve(g_s), s	4.8	4.5	6.8	3.3	0.0	8.9	3.5	5.9	6.0	1.9	12.4	12.4
Cycle Q Clear(g_c), s	4.8	4.5	6.8	3.3	0.0	8.9	3.5	5.9	6.0	1.9	12.4	12.4
Prop In Lane	1.00		1.00	1.00		0.30	1.00		0.21	1.00		0.36
Lane Grp Cap(c), veh/h	358	423	361	421	0	362	140	594	603	74	523	517
V/C Ratio(X)	0.55	0.42	0.60	0.33	0.00	0.80	0.79	0.41	0.41	0.81	0.81	0.81
Avail Cap(c_a), veh/h	358	604	516	448	0	555	187	594	603	174	569	563
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.8	18.5	19.3	15.2	0.0	20.9	25.2	14.3	14.3	26.5	18.1	18.2
Incr Delay (d2), s/veh	1.1	0.7	1.6	0.2	0.0	4.8	10.5	0.6	0.6	7.4	8.5	8.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.8	1.9	2.4	1.2	0.0	3.7	1.7	2.2	2.2	0.9	5.7	5.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	16.9	19.1	20.9	15.4	0.0	25.7	35.6	15.0	15.0	33.9	26.7	26.8
LnGrp LOS	B	B	C	B	A	C	D	B	B	C	C	C
Approach Vol, veh/h		592			427			603			901	
Approach Delay, s/veh		19.0			22.4			18.7			27.2	
Approach LOS		B			C			B			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	6.4	23.7	8.6	17.1	8.5	21.5	9.5	16.2				
Change Period (Y+Rc), s	4.0	5.0	4.0	4.5	4.0	5.0	4.0	4.5				
Max Green Setting (Gmax), s	5.5	18.5	5.5	18.0	6.0	18.0	5.5	18.0				
Max Q Clear Time (g_c+I1), s	3.9	8.0	5.3	8.8	5.5	14.4	6.8	10.9				
Green Ext Time (p_c), s	0.0	2.9	0.0	1.2	0.0	2.1	0.0	0.9				
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay			22.5									
HCM 6th LOS			C									

Queues

28: Hungary Spring Road & E Parham Road

10/12/2021



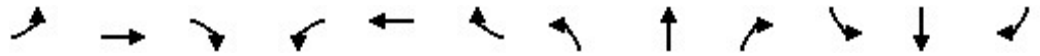
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	105	921	49	188	811	60	85	329	215	126	351	243
v/c Ratio	0.28	0.72	0.08	0.59	0.55	0.08	0.26	0.48	0.45	0.37	0.50	0.47
Control Delay	10.9	23.5	0.2	19.2	18.6	0.2	18.1	27.1	7.3	20.0	27.3	7.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.9	23.5	0.2	19.2	18.6	0.2	18.1	27.1	7.3	20.0	27.3	7.1
Queue Length 50th (ft)	20	175	0	37	142	0	25	67	0	38	72	0
Queue Length 95th (ft)	48	271	0	#112	224	0	53	104	50	74	110	52
Internal Link Dist (ft)		568			685			340			433	
Turn Bay Length (ft)	225		200	400		305	230		190	330		275
Base Capacity (vph)	376	1381	686	325	1479	715	321	1234	685	337	1270	730
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.67	0.07	0.58	0.55	0.08	0.26	0.27	0.31	0.37	0.28	0.33

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 28: Hungary Spring Road & E Parham Road

10/12/2021



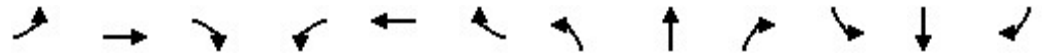
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	97	847	45	173	746	55	78	303	198	116	323	224
Future Volume (veh/h)	97	847	45	173	746	55	78	303	198	116	323	224
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1856	1841	1811	1841	1781	1870	1841	1826	1900	1885	1900
Adj Flow Rate, veh/h	105	921	49	188	811	60	85	329	215	126	351	243
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	3	4	6	4	8	2	4	5	0	1	0
Cap, veh/h	344	1230	544	345	1352	583	320	691	306	351	789	355
Arrive On Green	0.06	0.35	0.35	0.10	0.39	0.39	0.05	0.20	0.20	0.08	0.22	0.22
Sat Flow, veh/h	1781	3526	1559	1725	3497	1509	1781	3497	1547	1810	3582	1610
Grp Volume(v), veh/h	105	921	49	188	811	60	85	329	215	126	351	243
Grp Sat Flow(s),veh/h/ln	1781	1763	1559	1725	1749	1509	1781	1749	1547	1810	1791	1610
Q Serve(g_s), s	2.4	14.8	1.4	4.3	11.9	1.6	2.4	5.3	8.3	3.5	5.4	8.9
Cycle Q Clear(g_c), s	2.4	14.8	1.4	4.3	11.9	1.6	2.4	5.3	8.3	3.5	5.4	8.9
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	344	1230	544	345	1352	583	320	691	306	351	789	355
V/C Ratio(X)	0.30	0.75	0.09	0.55	0.60	0.10	0.27	0.48	0.70	0.36	0.44	0.69
Avail Cap(c_a), veh/h	418	1404	620	394	1480	638	377	1256	556	368	1286	578
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	12.8	18.4	14.0	13.5	15.7	12.6	19.0	22.8	23.9	18.4	21.6	22.9
Incr Delay (d2), s/veh	0.5	2.4	0.1	1.3	0.9	0.1	0.4	0.5	2.9	0.6	0.4	2.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	5.9	0.5	1.6	4.4	0.5	1.0	2.1	3.1	1.4	2.2	3.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	13.3	20.8	14.1	14.9	16.5	12.7	19.4	23.3	26.9	19.0	22.0	25.3
LnGrp LOS	B	C	B	B	B	B	B	C	C	B	C	C
Approach Vol, veh/h		1075			1059			629			720	
Approach Delay, s/veh		19.8			16.0			24.0			22.6	
Approach LOS		B			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.2	27.3	8.9	17.7	7.7	29.8	7.4	19.1				
Change Period (Y+Rc), s	4.0	5.0	4.0	5.0	4.0	5.0	4.0	5.0				
Max Green Setting (Gmax), s	8.0	25.5	5.5	23.0	6.4	27.1	5.5	23.0				
Max Q Clear Time (g_c+I1), s	6.3	16.8	5.5	10.3	4.4	13.9	4.4	10.9				
Green Ext Time (p_c), s	0.1	5.6	0.0	2.4	0.0	6.9	0.0	2.5				
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay				20.0								
HCM 6th LOS				B								



Queues

29: Woodman Road & E Parham Road

10/12/2021



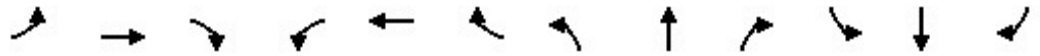
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	277	815	88	21	863	73	28	253	38	151	362	500
v/c Ratio	0.73	0.49	0.11	0.06	0.76	0.11	0.09	0.42	0.09	0.42	0.37	0.59
Control Delay	24.6	15.7	1.7	9.4	27.5	0.4	17.5	29.0	0.4	22.2	23.9	12.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	24.6	15.7	1.7	9.4	27.5	0.4	17.5	29.0	0.4	22.2	23.9	12.8
Queue Length 50th (ft)	55	101	0	4	171	0	9	55	0	51	66	85
Queue Length 95th (ft)	#184	226	13	15	273	0	25	88	0	93	120	220
Internal Link Dist (ft)		299			266			659			468	
Turn Bay Length (ft)	430		75	230		245	264		55	346		95
Base Capacity (vph)	391	1676	803	350	1239	683	319	901	544	358	1002	842
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.71	0.49	0.11	0.06	0.70	0.11	0.09	0.28	0.07	0.42	0.36	0.59

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 29: Woodman Road & E Parham Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↗	↘	↘	↗	↘	↘	↗	↘	↘	↗	↘
Traffic Volume (veh/h)	255	750	81	19	794	67	26	233	35	139	333	460
Future Volume (veh/h)	255	750	81	19	794	67	26	233	35	139	333	460
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1841	1811	1811	1841	1870	1900	1885	1841	1870	1885	1856
Adj Flow Rate, veh/h	277	815	0	21	863	0	28	253	0	151	362	500
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	3	4	6	6	4	2	0	1	4	2	1	3
Cap, veh/h	386	1498		292	1098		262	745		423	991	638
Arrive On Green	0.13	0.43	0.00	0.01	0.31	0.00	0.02	0.21	0.00	0.09	0.28	0.28
Sat Flow, veh/h	1767	3497	1535	1725	3497	1585	1810	3582	1560	1781	3582	1572
Grp Volume(v), veh/h	277	815	0	21	863	0	28	253	0	151	362	500
Grp Sat Flow(s),veh/h/ln	1767	1749	1535	1725	1749	1585	1810	1791	1560	1781	1791	1572
Q Serve(g_s), s	6.9	12.2	0.0	0.6	15.8	0.0	0.9	4.2	0.0	4.4	5.7	19.5
Cycle Q Clear(g_c), s	6.9	12.2	0.0	0.6	15.8	0.0	0.9	4.2	0.0	4.4	5.7	19.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	386	1498		292	1098		262	745		423	991	638
V/C Ratio(X)	0.72	0.54		0.07	0.79		0.11	0.34		0.36	0.37	0.78
Avail Cap(c_a), veh/h	435	1539		402	1266		371	915		446	991	638
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.2	15.0	0.0	16.2	22.0	0.0	21.4	23.8	0.0	18.0	20.5	18.3
Incr Delay (d2), s/veh	4.9	0.5	0.0	0.1	3.3	0.0	0.2	0.3	0.0	0.5	0.2	6.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.0	4.5	0.0	0.2	6.5	0.0	0.4	1.7	0.0	1.8	2.3	7.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	20.1	15.5	0.0	16.3	25.3	0.0	21.6	24.0	0.0	18.5	20.7	24.6
LnGrp LOS	C	B		B	C		C	C		B	C	C
Approach Vol, veh/h		1092	A		884	A		281	A		1013	
Approach Delay, s/veh		16.7			25.1			23.8			22.3	
Approach LOS		B			C			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.1	28.1	10.1	19.2	5.0	36.2	5.3	24.0				
Change Period (Y+Rc), s	4.0	6.0	4.0	4.5	4.0	6.0	4.0	4.5				
Max Green Setting (Gmax), s	11.0	25.5	7.0	18.0	5.5	31.0	5.5	19.5				
Max Q Clear Time (g_c+I1), s	8.9	17.8	6.4	6.2	2.6	14.2	2.9	21.5				
Green Ext Time (p_c), s	0.2	4.3	0.0	1.2	0.0	7.0	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay	21.3
HCM 6th LOS	C

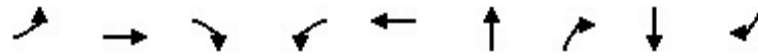
Notes

Unsignalized Delay for [NBR, EBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Queues

30: Bethlehem Road & Glenside Drive

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT	SBR
Lane Group Flow (vph)	62	683	87	118	903	101	71	138	112
v/c Ratio	0.28	0.70	0.16	0.51	0.65	0.51	0.22	0.76	0.18
Control Delay	21.5	32.4	3.2	27.7	29.3	42.8	3.2	55.8	4.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	21.5	32.4	3.2	27.7	29.3	42.8	3.2	55.8	4.8
Queue Length 50th (ft)	20	173	0	40	154	50	0	64	0
Queue Length 95th (ft)	50	#260	20	#85	216	98	11	#180	32
Internal Link Dist (ft)		521			373	430		331	
Turn Bay Length (ft)	275			250			130		610
Base Capacity (vph)	224	999	542	233	1426	340	462	192	665
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.68	0.16	0.51	0.63	0.30	0.15	0.72	0.17

























Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 30: Bethlehem Road & Glenside Drive

10/12/2021

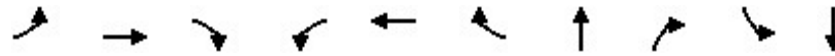
												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	57	628	80	109	797	34	51	42	65	38	89	103
Future Volume (vph)	57	628	80	109	797	34	51	42	65	38	89	103
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.5	4.5	4.0	4.5			4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91			1.00	1.00		1.00	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00	1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00	1.00		1.00	1.00
Frt	1.00	1.00	0.85	1.00	0.99			1.00	0.85		1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00			0.97	1.00		0.99	1.00
Satd. Flow (prot)	1805	3539	1615	1752	5008			1820	1538		1821	1583
Flt Permitted	0.19	1.00	1.00	0.21	1.00			0.76	1.00		0.28	1.00
Satd. Flow (perm)	352	3539	1615	389	5008			1414	1538		520	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	62	683	87	118	866	37	55	46	71	41	97	112
RTOR Reduction (vph)	0	0	63	0	5	0	0	0	63	0	0	73
Lane Group Flow (vph)	62	683	24	118	898	0	0	101	8	0	138	39
Confl. Peds. (#/hr)	1					1						
Heavy Vehicles (%)	0%	2%	0%	3%	3%	0%	3%	0%	5%	0%	4%	2%
Turn Type	pm+pt	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	1	6		5	2			4			3	
Permitted Phases	6		6	2			4		4	3		3
Actuated Green, G (s)	25.7	21.6	21.6	25.7	21.6			9.0	9.0		27.4	27.4
Effective Green, g (s)	25.7	21.6	21.6	25.7	21.6			9.0	9.0		27.4	27.4
Actuated g/C Ratio	0.32	0.27	0.27	0.32	0.27			0.11	0.11		0.34	0.34
Clearance Time (s)	4.0	4.5	4.5	4.0	4.5			4.5	4.5		4.5	4.5
Vehicle Extension (s)	2.0	4.5	4.5	2.0	4.5			3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	188	960	438	195	1358			159	173		178	544
v/s Ratio Prot	0.02	c0.19		c0.03	0.18							
v/s Ratio Perm	0.09		0.01	0.16				c0.07	0.01		c0.27	0.02
v/c Ratio	0.33	0.71	0.05	0.61	0.66			0.64	0.05		0.78	0.07
Uniform Delay, d1	19.4	26.2	21.4	20.3	25.8			33.7	31.5		23.3	17.5
Progression Factor	1.00	1.00	1.00	1.00	1.00			1.00	1.00		1.00	1.00
Incremental Delay, d2	0.4	2.9	0.1	3.6	1.5			8.0	0.1		18.8	0.1
Delay (s)	19.8	29.1	21.5	23.9	27.2			41.8	31.6		42.1	17.6
Level of Service	B	C	C	C	C			D	C		D	B
Approach Delay (s)		27.6			26.8			37.6			31.1	
Approach LOS		C			C			D			C	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			28.4			HCM 2000 Level of Service			C			
HCM 2000 Volume to Capacity ratio			0.72									
Actuated Cycle Length (s)			79.6			Sum of lost time (s)			17.5			
Intersection Capacity Utilization			47.7%			ICU Level of Service			A			
Analysis Period (min)			15									

c Critical Lane Group

Queues

31: Hermitage Road & Hilliard Road / Glenside Drive

10/12/2021



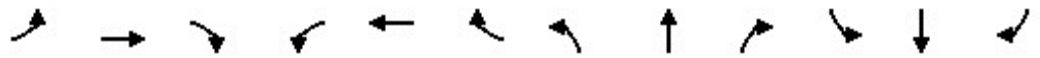
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	77	586	12	18	952	512	100	39	428	211
v/c Ratio	0.80	0.44	0.02	0.19	0.89	0.62	0.52	0.13	0.91	0.42
Control Delay	99.5	26.4	0.1	52.9	46.6	6.4	53.1	0.8	63.3	22.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	99.5	26.4	0.1	52.9	46.6	6.4	53.1	0.8	63.3	22.7
Queue Length 50th (ft)	51	136	0	12	313	0	63	0	273	69
Queue Length 95th (ft)	#142	234	0	36	#461	84	117	0	#486	145
Internal Link Dist (ft)		315			426		221			272
Turn Bay Length (ft)			35	415		415		45	215	
Base Capacity (vph)	96	1338	656	94	1065	825	316	397	469	501
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.80	0.44	0.02	0.19	0.89	0.62	0.32	0.10	0.91	0.42

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 31: Hermitage Road & Hilliard Road / Glenside Drive

10/12/2021

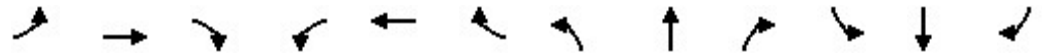


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	71	539	11	17	876	471	24	68	36	394	69	125
Future Volume (vph)	71	539	11	17	876	471	24	68	36	394	69	125
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.5	5.5	5.5	6.2	5.6	5.6		6.9	6.9	8.1	8.1	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00		1.00	1.00	1.00	1.00	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98		1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85		1.00	0.85	1.00	0.90	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00		0.99	1.00	0.95	1.00	
Satd. Flow (prot)	1656	3471	1417	1805	3471	1534		1809	1482	1770	1672	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00		0.99	1.00	0.95	1.00	
Satd. Flow (perm)	1656	3471	1417	1805	3471	1534		1809	1482	1770	1672	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	77	586	12	18	952	512	26	74	39	428	75	136
RTOR Reduction (vph)	0	0	8	0	0	342	0	0	35	0	59	0
Lane Group Flow (vph)	77	586	4	18	952	170	0	100	4	428	152	0
Confl. Peds. (#/hr)	1					1						
Heavy Vehicles (%)	9%	4%	14%	0%	4%	3%	0%	5%	9%	2%	2%	3%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Split	NA	Perm	Split	NA	
Protected Phases	1	6		5	2		3	3		4	4	
Permitted Phases			6			2			3			
Actuated Green, G (s)	6.0	39.7	39.7	2.1	35.4	35.4		11.0	11.0	27.3	27.3	
Effective Green, g (s)	6.0	39.7	39.7	2.1	35.4	35.4		11.0	11.0	27.3	27.3	
Actuated g/C Ratio	0.06	0.37	0.37	0.02	0.33	0.33		0.10	0.10	0.26	0.26	
Clearance Time (s)	6.5	5.5	5.5	6.2	5.6	5.6		6.9	6.9	8.1	8.1	
Vehicle Extension (s)	3.0	6.0	6.0	3.0	6.0	6.0		3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	93	1290	526	35	1150	508		186	152	452	427	
v/s Ratio Prot	c0.05	c0.17		0.01	c0.27			c0.06		c0.24	0.09	
v/s Ratio Perm			0.00			0.11			0.00			
v/c Ratio	0.83	0.45	0.01	0.51	0.83	0.33		0.54	0.03	0.95	0.36	
Uniform Delay, d1	49.9	25.4	21.1	51.8	32.9	26.8		45.5	43.1	39.0	32.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	
Incremental Delay, d2	42.7	0.7	0.0	12.2	5.9	1.1		3.0	0.1	29.0	0.5	
Delay (s)	92.6	26.1	21.2	64.0	38.8	27.9		48.5	43.2	68.0	33.1	
Level of Service	F	C	C	E	D	C		D	D	E	C	
Approach Delay (s)		33.6			35.3			47.0			56.5	
Approach LOS		C			D			D			E	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			40.1				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			0.84									
Actuated Cycle Length (s)			106.8				Sum of lost time (s)		27.1			
Intersection Capacity Utilization			73.7%				ICU Level of Service		D			
Analysis Period (min)			15									
c Critical Lane Group												

Queues

32: Lakeside Avenue & Hilliard Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	211	341	489	51	393	12	450	87	45	7	161	617
v/c Ratio	0.90	0.35	0.46	0.45	0.64	0.03	0.67	0.24	0.10	0.02	0.37	0.85
Control Delay	92.1	38.8	4.7	72.1	53.2	0.1	52.1	44.7	0.5	42.0	45.8	41.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	92.1	38.8	4.7	72.1	53.2	0.1	52.1	44.7	0.5	42.0	45.8	41.1
Queue Length 50th (ft)	172	122	34	41	158	0	174	60	0	4	112	393
Queue Length 95th (ft)	#360	174	99	90	220	0	247	115	0	19	198	#710
Internal Link Dist (ft)		216			777			626			561	
Turn Bay Length (ft)	150		230	115		200	130		90	265		
Base Capacity (vph)	235	1113	1095	132	877	539	815	447	515	418	432	728
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.90	0.31	0.45	0.39	0.45	0.02	0.55	0.19	0.09	0.02	0.37	0.85

























Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 32: Lakeside Avenue & Hilliard Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	194	314	450	47	362	11	414	80	41	6	148	568
Future Volume (vph)	194	314	450	47	362	11	414	80	41	6	148	568
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.2	5.5	5.5	7.3	6.1	6.1	8.3	8.3	8.3	9.1	9.1	9.1
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00	1.00	1.00	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1752	3374	1583	1719	3374	1594	3400	1863	1455	1805	1863	1568
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1752	3374	1583	1719	3374	1594	3400	1863	1455	1805	1863	1568
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	211	341	489	51	393	12	450	87	45	7	161	617
RTOR Reduction (vph)	0	0	193	0	0	10	0	0	34	0	0	52
Lane Group Flow (vph)	211	341	296	51	393	2	450	87	11	7	161	565
Confl. Peds. (#/hr)	1					1						
Heavy Vehicles (%)	3%	7%	2%	5%	7%	0%	3%	2%	11%	0%	2%	3%
Turn Type	Prot	NA	pt+ov	Prot	NA	Perm	Split	NA	pt+ov	Split	NA	pt+ov
Protected Phases	1	6	6 3	5	2		3	3	3 5	4	4	4 1
Permitted Phases						2						
Actuated Green, G (s)	16.9	35.9	66.4	7.0	24.5	24.5	25.0	25.0	32.0	29.2	29.2	55.2
Effective Green, g (s)	16.9	35.9	66.4	7.0	24.5	24.5	25.0	25.0	32.0	29.2	29.2	55.2
Actuated g/C Ratio	0.13	0.28	0.52	0.05	0.19	0.19	0.20	0.20	0.25	0.23	0.23	0.43
Clearance Time (s)	8.2	5.5		7.3	6.1	6.1	8.3	8.3		9.1	9.1	
Vehicle Extension (s)	3.0	5.0		3.0	5.0	5.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	232	951	825	94	649	306	667	365	365	414	427	679
v/s Ratio Prot	c0.12	0.10	0.19	0.03	c0.12		c0.13	0.05	0.01	0.00	0.09	c0.36
v/s Ratio Perm						0.00						
v/c Ratio	0.91	0.36	0.36	0.54	0.61	0.01	0.67	0.24	0.03	0.02	0.38	0.83
Uniform Delay, d1	54.4	36.5	17.9	58.6	47.0	41.6	47.4	43.1	36.0	37.9	41.4	32.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	35.2	0.5	0.6	6.3	2.4	0.0	2.7	0.3	0.0	0.0	0.6	8.6
Delay (s)	89.6	37.0	18.5	64.9	49.3	41.6	50.1	43.5	36.0	38.0	41.9	40.6
Level of Service	F	D	B	E	D	D	D	D	D	D	D	D
Approach Delay (s)		39.0			50.9			48.0			40.8	
Approach LOS		D			D			D			D	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			43.2				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			0.82									
Actuated Cycle Length (s)			127.3				Sum of lost time (s)		31.7			
Intersection Capacity Utilization			77.1%				ICU Level of Service		D			
Analysis Period (min)			15									

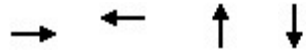
c Critical Lane Group



Queues

33: Hermitage Road & Dumbarton Road

















10/12/2021



Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	663	1218	73	109
v/c Ratio	0.31	0.53	0.26	0.38
Control Delay	4.6	6.1	14.6	15.1
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	4.6	6.1	14.6	15.1
Queue Length 50th (ft)	31	71	12	15
Queue Length 95th (ft)	65	143	36	46
Internal Link Dist (ft)	196	212	242	190
Turn Bay Length (ft)				
Base Capacity (vph)	2106	2291	748	730
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.31	0.53	0.10	0.15
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
 33: Hermitage Road & Dumbarton Road

10/12/2021

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	32	558	19	9	1089	22	41	11	15	37	11	52	
Future Volume (vph)	32	558	19	9	1089	22	41	11	15	37	11	52	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		4.5			4.5			4.5			4.5		
Lane Util. Factor		0.95			0.95			1.00			1.00		
Frb, ped/bikes		1.00			1.00			1.00			0.99		
Flpb, ped/bikes		1.00			1.00			1.00			1.00		
Frt		1.00			1.00			0.97			0.93		
Flt Protected		1.00			1.00			0.97			0.98		
Satd. Flow (prot)		3539			3522			1784			1719		
Flt Permitted		0.87			0.95			0.84			0.85		
Satd. Flow (perm)		3075			3348			1543			1485		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	35	607	21	10	1184	24	45	12	16	40	12	57	
RTOR Reduction (vph)	0	3	0	0	2	0	0	14	0	0	30	0	
Lane Group Flow (vph)	0	660	0	0	1216	0	0	59	0	0	79	0	
Confl. Peds. (#/hr)			2	2			7					7	
Heavy Vehicles (%)	5%	1%	0%	0%	2%	10%	0%	0%	0%	0%	0%	0%	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA		
Protected Phases		4			4			2			2		
Permitted Phases	4			4			2			2			
Actuated Green, G (s)		28.3			28.3			6.4			6.4		
Effective Green, g (s)		28.3			28.3			6.4			6.4		
Actuated g/C Ratio		0.65			0.65			0.15			0.15		
Clearance Time (s)		4.5			4.5			4.5			4.5		
Vehicle Extension (s)		4.0			4.0			3.0			3.0		
Lane Grp Cap (vph)		1991			2168			225			217		
v/s Ratio Prot													
v/s Ratio Perm		0.21			c0.36			0.04			c0.05		
v/c Ratio		0.33			0.56			0.26			0.36		
Uniform Delay, d1		3.5			4.3			16.6			16.8		
Progression Factor		1.00			1.00			1.00			1.00		
Incremental Delay, d2		0.1			0.4			0.6			1.0		
Delay (s)		3.6			4.7			17.2			17.9		
Level of Service		A			A			B			B		
Approach Delay (s)		3.6			4.7			17.2			17.9		
Approach LOS		A			A			B			B		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			5.5									HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio			0.52										
Actuated Cycle Length (s)			43.7									Sum of lost time (s)	9.0
Intersection Capacity Utilization			56.1%									ICU Level of Service	B
Analysis Period (min)			15										
c Critical Lane Group													

Queues

34: Lakeside Avenue & Dumbarton Road

10/12/2021



Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	359	326	1078	171	183	435	114	868
v/c Ratio	0.73	0.65	0.97	0.29	0.93	0.44	0.66	0.95
Control Delay	66.3	11.8	68.0	10.1	108.9	43.8	78.8	70.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.3	11.8	68.0	10.1	108.9	43.8	78.8	70.2
Queue Length 50th (ft)	166	0	510	19	168	170	101	406
Queue Length 95th (ft)	222	92	#696	78	#332	241	171	#571
Internal Link Dist (ft)	371		350			338		383
Turn Bay Length (ft)		195		300	210		265	
Base Capacity (vph)	620	546	1110	599	197	983	221	913
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.58	0.60	0.97	0.29	0.93	0.44	0.52	0.95

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 34: Lakeside Avenue & Dumbarton Road

10/12/2021

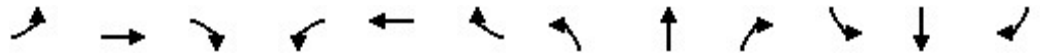
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	65	265	300	161	831	157	168	363	37	105	688	110
Future Volume (vph)	65	265	300	161	831	157	168	363	37	105	688	110
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.5	5.5		5.5	5.5	5.5	6.0		5.5	6.0	
Lane Util. Factor		0.95	1.00		0.95	1.00	1.00	0.95		1.00	0.95	
Frbp, ped/bikes		1.00	0.99		1.00	0.99	1.00	1.00		1.00	1.00	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	0.99		1.00	0.98	
Flt Protected		0.99	1.00		0.99	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		3471	1562		3477	1577	1719	3511		1770	3450	
Flt Permitted		0.99	1.00		0.99	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)		3471	1562		3477	1577	1719	3511		1770	3450	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	71	288	326	175	903	171	183	395	40	114	748	120
RTOR Reduction (vph)	0	0	279	0	0	96	0	5	0	0	9	0
Lane Group Flow (vph)	0	359	47	0	1078	75	183	430	0	114	859	0
Confl. Peds. (#/hr)	1		1	1		1	1		1	1		1
Heavy Vehicles (%)	3%	3%	2%	3%	3%	1%	5%	1%	4%	2%	2%	4%
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases			4			3						
Actuated Green, G (s)		20.0	20.0		44.7	44.7	16.1	39.0		13.8	36.7	
Effective Green, g (s)		20.0	20.0		44.7	44.7	16.1	39.0		13.8	36.7	
Actuated g/C Ratio		0.14	0.14		0.32	0.32	0.12	0.28		0.10	0.26	
Clearance Time (s)		5.5	5.5		5.5	5.5	5.5	6.0		5.5	6.0	
Vehicle Extension (s)		2.5	2.5		2.5	2.5	3.0	6.0		3.0	6.0	
Lane Grp Cap (vph)		495	223		1110	503	197	978		174	904	
v/s Ratio Prot		c0.10			c0.31		c0.11	0.12		0.06	c0.25	
v/s Ratio Perm			0.03			0.05						
v/c Ratio		0.73	0.21		0.97	0.15	0.93	0.44		0.66	0.95	
Uniform Delay, d1		57.4	53.0		47.0	34.1	61.4	41.5		60.8	50.8	
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2		4.9	0.3		20.3	0.1	43.9	0.9		8.6	19.7	
Delay (s)		62.3	53.4		67.3	34.2	105.3	42.4		69.4	70.4	
Level of Service		E	D		E	C	F	D		E	E	
Approach Delay (s)		58.0			62.8			61.0			70.3	
Approach LOS		E			E			E			E	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			63.6				HCM 2000 Level of Service			E		
HCM 2000 Volume to Capacity ratio			0.92									
Actuated Cycle Length (s)			140.0				Sum of lost time (s)			22.5		
Intersection Capacity Utilization			87.8%				ICU Level of Service			E		
Analysis Period (min)			15									

c Critical Lane Group

Queues

1: W Broad Street & Gaskins Road

10/12/2021




































Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	522	783	352	116	574	493	528	1517	97	618	1386	664
v/c Ratio	1.24	1.24	0.65	0.78	1.16	1.05	0.87	0.74	0.14	1.13	0.72	0.88
Control Delay	168.7	160.3	13.7	84.0	135.2	77.2	61.7	32.2	3.2	123.3	33.2	34.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	168.7	160.3	13.7	84.0	135.2	77.2	61.7	32.2	3.2	123.3	33.2	34.7
Queue Length 50th (ft)	~245	~380	25	85	~262	~203	194	343	0	~271	319	302
Queue Length 95th (ft)	#354	#504	122	#178	#376	#413	#275	401	24	#385	375	#544
Internal Link Dist (ft)		695			440			948			564	
Turn Bay Length (ft)	550		550	270		270	550		950	425		430
Base Capacity (vph)	421	632	539	155	496	468	632	2047	707	547	1931	756
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.24	1.24	0.65	0.75	1.16	1.05	0.84	0.74	0.14	1.13	0.72	0.88

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 1: W Broad Street & Gaskins Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 			 		 	  		 	  	
Traffic Volume (veh/h)	496	744	334	110	545	468	502	1441	92	587	1317	631
Future Volume (veh/h)	496	744	334	110	545	468	502	1441	92	587	1317	631
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1885	1870	1900	1900	1885	1885	1870
Adj Flow Rate, veh/h	522	783	0	116	574	493	528	1517	97	618	1386	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	1	2	0	0	1	1	2
Cap, veh/h	424	652		142	499	224	589	2048	636	550	1967	
Arrive On Green	0.12	0.18	0.00	0.08	0.14	0.14	0.17	0.39	0.39	0.16	0.38	0.00
Sat Flow, veh/h	3456	3554	1585	1781	3554	1598	3456	5187	1610	3483	5147	1585
Grp Volume(v), veh/h	522	783	0	116	574	493	528	1517	97	618	1386	0
Grp Sat Flow(s),veh/h/ln	1728	1777	1585	1781	1777	1598	1728	1729	1610	1742	1716	1585
Q Serve(g_s), s	14.0	20.9	0.0	7.3	16.0	16.0	17.1	28.5	4.4	18.0	26.0	0.0
Cycle Q Clear(g_c), s	14.0	20.9	0.0	7.3	16.0	16.0	17.1	28.5	4.4	18.0	26.0	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	424	652		142	499	224	589	2048	636	550	1967	
V/C Ratio(X)	1.23	1.20		0.82	1.15	2.20	0.90	0.74	0.15	1.12	0.70	
Avail Cap(c_a), veh/h	424	652		156	499	224	637	2048	636	550	1967	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	50.0	46.5	0.0	51.6	49.0	49.0	46.3	29.5	22.2	48.0	29.8	0.0
Incr Delay (d2), s/veh	122.6	104.7	0.0	24.7	88.9	553.7	14.4	2.5	0.5	77.1	2.1	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	13.2	18.6	0.0	4.2	13.2	40.8	8.5	12.1	1.8	13.7	10.9	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	172.6	151.2	0.0	76.4	137.9	602.7	60.7	32.0	22.7	125.1	31.9	0.0
LnGrp LOS	F	F		E	F	F	E	C	C	F	C	
Approach Vol, veh/h		1305	A		1183			2142			2004	A
Approach Delay, s/veh		159.8			325.6			38.6			60.7	
Approach LOS		F			F			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	24.4	49.6	19.0	21.0	23.0	51.0	14.1	25.9				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	21.0	42.0	14.0	16.0	18.0	45.0	10.0	20.0				
Max Q Clear Time (g_c+I1), s	19.1	28.0	16.0	18.0	20.0	30.5	9.3	22.9				
Green Ext Time (p_c), s	0.4	11.1	0.0	0.0	0.0	12.3	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay	120.3
HCM 6th LOS	F

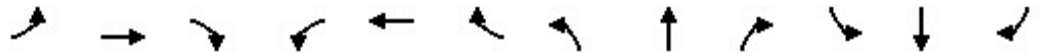
Notes

Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Queues

2: W Broad Street & Pemberton Road / Springfield Road

10/12/2021



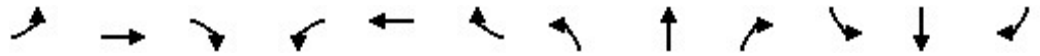
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	102	242	214	555	307	91	219	1851	577	99	1585	111
v/c Ratio	0.75	0.89	0.36	0.92	0.67	0.18	0.91	0.85	0.53	0.80	0.87	0.16
Control Delay	83.7	81.6	13.9	68.9	47.3	0.7	88.1	34.4	9.6	93.0	40.5	0.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	83.7	81.6	13.9	68.9	47.3	0.7	88.1	34.4	9.6	93.0	40.5	0.5
Queue Length 50th (ft)	74	176	46	208	204	0	160	444	152	73	398	0
Queue Length 95th (ft)	#161	#318	109	#308	302	0	#301	513	237	#167	465	0
Internal Link Dist (ft)		380			538			1004			707	
Turn Bay Length (ft)	150		350	460			400			270		280
Base Capacity (vph)	141	277	605	608	462	522	245	2172	1096	125	1826	703
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.72	0.87	0.35	0.91	0.66	0.17	0.89	0.85	0.53	0.79	0.87	0.16

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 2: W Broad Street & Pemberton Road / Springfield Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	99	235	208	538	298	88	212	1795	560	96	1537	108
Future Volume (veh/h)	99	235	208	538	298	88	212	1795	560	96	1537	108
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1870	1870	1885	1885	1885	1856	1870	1885	1885	1870	1870
Adj Flow Rate, veh/h	102	242	214	555	307	91	219	1851	577	99	1585	111
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	1	2	2	1	1	1	3	2	1	1	2	2
Cap, veh/h	127	272	452	607	470	398	246	2180	961	123	1821	565
Arrive On Green	0.07	0.15	0.15	0.17	0.25	0.25	0.14	0.43	0.43	0.07	0.36	0.36
Sat Flow, veh/h	1795	1870	1585	3483	1885	1598	1767	5106	1598	1795	5106	1585
Grp Volume(v), veh/h	102	242	214	555	307	91	219	1851	577	99	1585	111
Grp Sat Flow(s),veh/h/ln	1795	1870	1585	1742	1885	1598	1767	1702	1598	1795	1702	1585
Q Serve(g_s), s	6.4	14.5	12.7	17.8	16.6	5.2	13.9	37.1	25.7	6.2	33.0	5.5
Cycle Q Clear(g_c), s	6.4	14.5	12.7	17.8	16.6	5.2	13.9	37.1	25.7	6.2	33.0	5.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	127	272	452	607	470	398	246	2180	961	123	1821	565
V/C Ratio(X)	0.80	0.89	0.47	0.91	0.65	0.23	0.89	0.85	0.60	0.80	0.87	0.20
Avail Cap(c_a), veh/h	142	279	457	611	470	398	248	2180	961	126	1821	565
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	52.2	47.8	33.7	46.2	38.4	34.1	48.2	29.4	14.2	52.3	34.2	25.4
Incr Delay (d2), s/veh	24.1	27.1	0.8	18.1	3.2	0.3	29.7	4.4	2.8	28.6	6.0	0.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.7	8.8	5.0	9.2	8.1	2.0	8.1	15.7	9.5	3.8	14.4	2.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	76.3	74.8	34.5	64.3	41.6	34.3	77.9	33.7	17.0	80.9	40.3	26.2
LnGrp LOS	E	E	C	E	D	C	E	C	B	F	D	C
Approach Vol, veh/h		558			953			2647			1795	
Approach Delay, s/veh		59.6			54.2			33.7			41.6	
Approach LOS		E			D			C			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	20.9	46.6	24.9	21.6	12.8	54.7	13.1	33.4				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	16.0	40.0	20.0	17.0	8.0	48.0	9.0	28.0				
Max Q Clear Time (g_c+I1), s	15.9	35.0	19.8	16.5	8.2	39.1	8.4	18.6				
Green Ext Time (p_c), s	0.0	4.0	0.0	0.1	0.0	7.7	0.0	1.4				
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay			41.8									
HCM 6th LOS			D									



Queues

3: W Broad Street & West End Drive / Commercial Ent.

10/12/2021



Lane Group	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	54	45	144	141	214	24	2164	492	258	1740
v/c Ratio	0.43	0.17	0.68	0.66	0.55	0.24	0.91	0.44	0.88	0.55
Control Delay	60.5	1.4	63.0	61.8	11.7	56.1	36.7	2.2	76.2	16.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.5	1.4	63.0	61.8	11.7	56.1	36.7	2.2	76.2	16.2
Queue Length 50th (ft)	39	0	105	103	0	17	~586	10	184	308
Queue Length 95th (ft)	79	0	178	175	69	44	#708	33	#348	393
Internal Link Dist (ft)	349			285			640			671
Turn Bay Length (ft)					160	65		215	450	
Base Capacity (vph)	177	306	243	244	412	170	2378	1149	294	3148
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.15	0.59	0.58	0.52	0.14	0.91	0.43	0.88	0.55

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.
























# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 3: W Broad Street & West End Drive / Commercial Ent.

10/12/2021

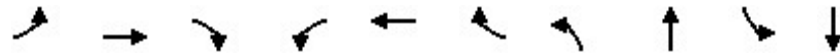
													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	30	22	44	274	3	208	23	2099	477	250	1656	32	
Future Volume (vph)	30	22	44	274	3	208	23	2099	477	250	1656	32	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0	5.0	5.0	5.0	5.0	6.0	5.0	5.0	6.0		
Lane Util. Factor		1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91		
Frbp, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Frt		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00		
Flt Protected		0.97	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (prot)		1836	1568	1681	1688	1583	1770	5085	1568	1736	5118		
Flt Permitted		0.97	1.00	0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (perm)		1836	1568	1681	1688	1583	1770	5085	1568	1736	5118		
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	31	23	45	282	3	214	24	2164	492	258	1707	33	
RTOR Reduction (vph)	0	0	42	0	0	187	0	0	181	0	1	0	
Lane Group Flow (vph)	0	54	3	144	141	27	24	2164	311	258	1739	0	
Confl. Peds. (#/hr)							2					2	
Heavy Vehicles (%)	1%	0%	3%	2%	0%	2%	2%	2%	3%	4%	1%	2%	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	pm+ov	Prot	NA		
Protected Phases	3	3		4	4		1	6	4	5	2		
Permitted Phases			3			4			6				
Actuated Green, G (s)		6.9	6.9	14.5	14.5	14.5	4.5	52.3	66.8	19.3	67.1		
Effective Green, g (s)		6.9	6.9	14.5	14.5	14.5	4.5	52.3	66.8	19.3	67.1		
Actuated g/C Ratio		0.06	0.06	0.13	0.13	0.13	0.04	0.46	0.59	0.17	0.59		
Clearance Time (s)		5.0	5.0	5.0	5.0	5.0	5.0	6.0	5.0	5.0	6.0		
Vehicle Extension (s)		2.0	2.0	2.0	2.0	2.0	2.5	4.0	2.0	2.5	4.0		
Lane Grp Cap (vph)		111	94	213	214	201	69	2332	918	293	3012		
v/s Ratio Prot		c0.03		c0.09	0.08		0.01	c0.43	0.04	c0.15	0.34		
v/s Ratio Perm			0.00			0.02			0.16				
v/c Ratio		0.49	0.03	0.68	0.66	0.14	0.35	0.93	0.34	0.88	0.58		
Uniform Delay, d1		51.8	50.4	47.5	47.4	44.2	53.3	29.1	12.2	46.2	14.6		
Progression Factor		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2		1.2	0.0	6.5	5.5	0.1	2.2	8.0	0.1	24.9	0.8		
Delay (s)		53.1	50.4	54.0	52.9	44.3	55.5	37.1	12.3	71.1	15.4		
Level of Service		D	D	D	D	D	E	D	B	E	B		
Approach Delay (s)		51.9			49.5			32.7			22.6		
Approach LOS		D			D			C			C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			30.8		HCM 2000 Level of Service					C			
HCM 2000 Volume to Capacity ratio			0.84										
Actuated Cycle Length (s)			114.0		Sum of lost time (s)					21.0			
Intersection Capacity Utilization			82.1%		ICU Level of Service					E			
Analysis Period (min)			15										

c Critical Lane Group

Queues

4: W Broad Street & N Parham Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	90	672	103	170	714	589	261	1702	472	1555
v/c Ratio	0.57	1.03	0.24	0.91	0.93	0.88	0.92	1.09	1.22	0.84
Control Delay	63.3	89.6	1.6	98.0	64.5	29.9	83.4	88.9	157.8	38.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	63.3	89.6	1.6	98.0	64.5	29.9	83.4	88.9	157.8	38.3
Queue Length 50th (ft)	64	~278	0	125	~285	140	189	~513	~425	380
Queue Length 95th (ft)	114	#398	3	#258	#466	#389	#339	#612	#630	445
Internal Link Dist (ft)		487			952			657		412
Turn Bay Length (ft)	230		55	475		875	475		275	
Base Capacity (vph)	248	651	425	186	768	668	295	1562	388	1844
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.36	1.03	0.24	0.91	0.93	0.88	0.88	1.09	1.22	0.84

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

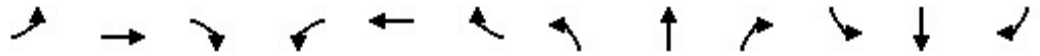
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 4: W Broad Street & N Parham Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑↑		↘	↑↑↑	
Traffic Volume (veh/h)	83	618	95	156	657	542	240	1423	143	434	1335	96
Future Volume (veh/h)	83	618	95	156	657	542	240	1423	143	434	1335	96
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1856	1870	1856	1870	1870	1885	1885	1870	1870	1870
Adj Flow Rate, veh/h	90	672	103	170	714	589	261	1547	155	472	1451	104
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	3	2	3	2	2	1	1	2	2	2
Cap, veh/h	114	655	289	188	795	357	288	1460	146	391	1773	127
Arrive On Green	0.06	0.18	0.18	0.11	0.23	0.23	0.16	0.31	0.31	0.22	0.36	0.36
Sat Flow, veh/h	1781	3554	1570	1781	3526	1583	1781	4755	476	1781	4863	349
Grp Volume(v), veh/h	90	672	103	170	714	589	261	1116	586	472	1016	539
Grp Sat Flow(s),veh/h/ln	1781	1777	1570	1781	1763	1583	1781	1716	1800	1781	1702	1808
Q Serve(g_s), s	5.7	21.0	6.5	10.8	22.4	25.7	16.4	35.0	35.0	25.0	30.8	30.8
Cycle Q Clear(g_c), s	5.7	21.0	6.5	10.8	22.4	25.7	16.4	35.0	35.0	25.0	30.8	30.8
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.26	1.00		0.19
Lane Grp Cap(c), veh/h	114	655	289	188	795	357	288	1053	552	391	1241	659
V/C Ratio(X)	0.79	1.03	0.36	0.91	0.90	1.65	0.91	1.06	1.06	1.21	0.82	0.82
Avail Cap(c_a), veh/h	250	655	289	188	795	357	297	1053	552	391	1241	659
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	52.6	46.5	40.6	50.4	42.9	44.1	46.9	39.5	39.5	44.5	32.8	32.8
Incr Delay (d2), s/veh	4.6	42.1	2.7	39.7	14.5	304.7	28.2	44.9	55.4	115.4	6.1	10.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.7	13.0	2.7	6.8	11.3	40.2	9.5	21.0	23.7	23.4	13.5	15.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	57.2	88.6	43.3	90.1	57.3	348.8	75.1	84.4	94.9	159.9	38.9	43.7
LnGrp LOS	E	F	D	F	E	F	E	F	F	F	D	D
Approach Vol, veh/h		865			1473			1963			2027	
Approach Delay, s/veh		79.9			177.7			86.3			68.3	
Approach LOS		E			F			F			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	23.4	47.6	12.3	30.7	30.0	41.0	17.0	26.0				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	19.0	41.0	16.0	17.0	25.0	35.0	12.0	21.0				
Max Q Clear Time (g_c+I1), s	18.4	32.8	7.7	27.7	27.0	37.0	12.8	23.0				
Green Ext Time (p_c), s	0.0	7.6	0.1	0.0	0.0	0.0	0.0	0.0				

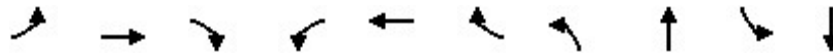
Intersection Summary

HCM 6th Ctrl Delay	100.9
HCM 6th LOS	F

Queues

5: W Broad Street & Hungary Spring Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	137	275	194	211	266	124	213	2151	21	474
v/c Ratio	0.66	1.25	0.37	0.82	0.99	0.34	1.03	0.84	0.21	0.24
Control Delay	63.8	186.3	6.5	73.2	101.5	5.5	120.1	28.2	55.6	23.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	63.8	186.3	6.5	73.2	101.5	5.5	120.1	28.2	55.6	23.3
Queue Length 50th (ft)	97	~252	0	152	197	0	~167	411	15	82
Queue Length 95th (ft)	#178	#420	54	#280	#367	28	#320	#684	41	109
Internal Link Dist (ft)		431			703			305		418
Turn Bay Length (ft)	225		350	325			275		175	
Base Capacity (vph)	209	220	520	256	269	363	207	2550	164	1972
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.66	1.25	0.37	0.82	0.99	0.34	1.03	0.84	0.13	0.24

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

























# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 5: W Broad Street & Hungary Spring Road

10/12/2021

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	130	261	184	200	253	118	202	1625	418	20	427	24	
Future Volume (vph)	130	261	184	200	253	118	202	1625	418	20	427	24	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	6.5	6.5	6.5	6.5	6.5	6.5	6.5	6.8		6.5	6.5		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91		1.00	0.91		
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00		1.00	1.00		
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00		
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.97		1.00	0.99		
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (prot)	1770	1863	1573	1770	1863	1561	1752	4905		1787	5041		
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (perm)	1770	1863	1573	1770	1863	1561	1752	4905		1787	5041		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Adj. Flow (vph)	137	275	194	211	266	124	213	1711	440	21	449	25	
RTOR Reduction (vph)	0	0	148	0	0	106	0	35	0	0	5	0	
Lane Group Flow (vph)	137	275	46	211	266	18	213	2116	0	21	469	0	
Confl. Peds. (#/hr)	1		1	1		1	2		1	1		2	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	3%	2%	3%	1%	2%	1%	
Turn Type	Split	NA	pm+ov	Split	NA	Perm	Prot	NA		Prot	NA		
Protected Phases	4	4		3	3		1	6		5	2		
Permitted Phases			4			3							
Actuated Green, G (s)	13.5	13.5	27.0	16.5	16.5	16.5	13.5	54.6		3.1	44.5		
Effective Green, g (s)	13.5	13.5	27.0	16.5	16.5	16.5	13.5	54.6		3.1	44.5		
Actuated g/C Ratio	0.12	0.12	0.24	0.14	0.14	0.14	0.12	0.48		0.03	0.39		
Clearance Time (s)	6.5	6.5	6.5	6.5	6.5	6.5	6.5	6.8		6.5	6.5		
Vehicle Extension (s)	3.0	3.0	2.5	3.0	3.0	3.0	2.5	4.0		2.5	4.0		
Lane Grp Cap (vph)	209	220	462	256	269	225	207	2349		48	1967		
v/s Ratio Prot	0.08	c0.15	0.01	0.12	c0.14		c0.12	c0.43		0.01	0.09		
v/s Ratio Perm			0.02			0.01							
v/c Ratio	0.66	1.25	0.10	0.82	0.99	0.08	1.03	0.90		0.44	0.24		
Uniform Delay, d1	48.0	50.2	34.0	47.3	48.7	42.2	50.2	27.2		54.6	23.4		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00		
Incremental Delay, d2	7.2	144.4	0.1	18.9	51.2	0.2	70.3	6.1		4.6	0.3		
Delay (s)	55.2	194.6	34.1	66.3	99.9	42.3	120.6	33.3		59.2	23.6		
Level of Service	E	F	C	E	F	D	F	C		E	C		
Approach Delay (s)		111.7			76.2			41.2			25.2		
Approach LOS		F			E			D			C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			54.9									HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio			1.02										
Actuated Cycle Length (s)			114.0									Sum of lost time (s)	26.3
Intersection Capacity Utilization			91.6%									ICU Level of Service	F
Analysis Period (min)			15										
c Critical Lane Group													

Queues

6: W Broad Street & Bethlehem Road / Ent. To Volvo

10/12/2021



Lane Group	EBT	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	11	101	169	5	2279	181	2069
v/c Ratio	0.12	0.81	0.60	0.06	0.71	0.80	0.53
Control Delay	49.2	94.4	19.2	53.2	15.5	73.3	5.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.2	94.4	19.2	53.2	15.5	73.3	5.2
Queue Length 50th (ft)	7	74	10	4	391	129	141
Queue Length 95th (ft)	26	#168	78	17	448	#230	279
Internal Link Dist (ft)	239	270			271		254
Turn Bay Length (ft)			95	200		175	
Base Capacity (vph)	152	129	290	162	3201	248	3931
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.78	0.58	0.03	0.71	0.73	0.53

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 6: W Broad Street & Bethlehem Road / Ent. To Volvo

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔	↔	↔	↑↑↑		↔	↑↑↑	
Traffic Volume (veh/h)	6	4	1	94	1	159	5	2047	95	170	1775	170
Future Volume (veh/h)	6	4	1	94	1	159	5	2047	95	170	1775	170
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1856	1796	1856	1856	1870	1856	1856
Adj Flow Rate, veh/h	6	4	1	100	1	169	5	2178	101	181	1888	181
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	3	7	3	3	2	3	3
Cap, veh/h	58	31	4	179	1	152	11	3203	148	210	3559	339
Arrive On Green	0.10	0.10	0.10	0.10	0.10	0.10	0.01	0.65	0.65	0.12	0.76	0.76
Sat Flow, veh/h	99	320	42	1201	12	1572	1711	4962	229	1781	4703	448
Grp Volume(v), veh/h	11	0	0	101	0	169	5	1479	800	181	1353	716
Grp Sat Flow(s),veh/h/ln	461	0	0	1213	0	1572	1711	1689	1814	1781	1689	1774
Q Serve(g_s), s	0.1	0.0	0.0	0.0	0.0	11.0	0.3	31.5	31.9	11.4	18.5	18.8
Cycle Q Clear(g_c), s	9.7	0.0	0.0	9.6	0.0	11.0	0.3	31.5	31.9	11.4	18.5	18.8
Prop In Lane	0.55		0.09	0.99		1.00	1.00		0.13	1.00		0.25
Lane Grp Cap(c), veh/h	93	0	0	180	0	152	11	2180	1171	210	2556	1343
V/C Ratio(X)	0.12	0.00	0.00	0.56	0.00	1.11	0.46	0.68	0.68	0.86	0.53	0.53
Avail Cap(c_a), veh/h	93	0	0	180	0	152	165	2180	1171	250	2556	1343
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	47.1	0.0	0.0	50.9	0.0	51.5	56.4	12.8	12.8	49.4	5.6	5.7
Incr Delay (d2), s/veh	0.4	0.0	0.0	3.3	0.0	106.9	20.3	1.7	3.2	21.5	0.8	1.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	0.0	0.0	3.0	0.0	8.8	0.2	11.5	13.0	6.3	5.7	6.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	47.5	0.0	0.0	54.2	0.0	158.4	76.7	14.5	16.1	70.9	6.4	7.2
LnGrp LOS	D	A	A	D	A	F	E	B	B	E	A	A
Approach Vol, veh/h		11			270			2284			2250	
Approach Delay, s/veh		47.5			119.4			15.2			11.8	
Approach LOS		D			F			B			B	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	5.7	92.3		16.0	18.4	79.6		16.0				
Change Period (Y+Rc), s	5.0	6.0		5.0	5.0	6.0		5.0				
Max Green Setting (Gmax), s	11.0	76.0		11.0	16.0	71.0		11.0				
Max Q Clear Time (g_c+I1), s	2.3	20.8		13.0	13.4	33.9		11.7				
Green Ext Time (p_c), s	0.0	38.6		0.0	0.1	31.0		0.0				

Intersection Summary

HCM 6th Ctrl Delay	19.5
HCM 6th LOS	B



Queues

7: W Broad Street & Glenside Drive

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	348	729	200	192	665	240	60	1731	422	318	1139	1001
v/c Ratio	1.25	1.24	0.13	0.73	1.27	0.40	0.47	1.03	0.51	1.23	0.75	1.01
Control Delay	177.5	163.6	0.2	63.5	177.7	20.8	62.3	68.2	9.6	174.9	32.4	47.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	177.5	163.6	0.2	63.5	177.7	20.8	62.3	68.2	9.6	174.9	32.4	47.6
Queue Length 50th (ft)	~349	~365	0	136	~324	87	43	~499	66	~288	378	~527
Queue Length 95th (ft)	#550	#493	0	#238	#443	158	87	#595	109	#467	476	#785
Internal Link Dist (ft)		738			586			849			590	
Turn Bay Length (ft)	650		410			230	775		425	485		
Base Capacity (vph)	279	586	1568	263	522	605	155	1678	823	258	1514	989
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.25	1.24	0.13	0.73	1.27	0.40	0.39	1.03	0.51	1.23	0.75	1.01

























Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 7: W Broad Street & Glenside Drive

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	398	647	194	186	645	233	58	1679	409	308	1105	971
Future Volume (vph)	398	647	194	186	645	233	58	1679	409	308	1105	971
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	4.0	6.0	6.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lane Util. Factor	0.91	0.91	1.00	1.00	0.95	1.00	1.00	0.91	1.00	1.00	0.95	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1595	3343	1568	1770	3505	1583	1770	5036	1583	1736	3539	1563
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1595	3343	1568	1770	3505	1583	1770	5036	1583	1736	3539	1563
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	410	667	200	192	665	240	60	1731	422	318	1139	1001
RTOR Reduction (vph)	0	0	0	0	0	54	0	0	60	0	0	326
Lane Group Flow (vph)	348	729	200	192	665	186	60	1731	362	318	1139	675
Confl. Peds. (#/hr)							1					1
Heavy Vehicles (%)	3%	3%	3%	2%	3%	2%	2%	3%	2%	4%	2%	2%
Turn Type	Split	NA	Free	Split	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	Perm
Protected Phases	3	3		4	4	5	1	6	4	5	2	
Permitted Phases			Free			4			6			2
Actuated Green, G (s)	20.0	20.0	114.0	17.0	17.0	34.0	7.2	38.0	55.0	17.0	47.8	47.8
Effective Green, g (s)	20.0	20.0	114.0	17.0	17.0	34.0	7.2	38.0	55.0	17.0	47.8	47.8
Actuated g/C Ratio	0.18	0.18	1.00	0.15	0.15	0.30	0.06	0.33	0.48	0.15	0.42	0.42
Clearance Time (s)	5.0	5.0		6.0	6.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Vehicle Extension (s)	2.5	2.5		2.5	2.5	2.5	2.5	5.0	2.5	2.5	5.0	5.0
Lane Grp Cap (vph)	279	586	1568	263	522	472	111	1678	763	258	1483	655
v/s Ratio Prot	c0.22	0.22		0.11	c0.19	0.06	0.03	0.34	0.07	c0.18	0.32	
v/s Ratio Perm			0.13			0.06			0.16			c0.43
v/c Ratio	1.25	1.24	0.13	0.73	1.27	0.39	0.54	1.03	0.48	1.23	0.77	1.03
Uniform Delay, d1	47.0	47.0	0.0	46.3	48.5	31.8	51.8	38.0	19.8	48.5	28.4	33.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	137.6	123.7	0.2	9.4	137.6	0.4	4.2	30.5	0.3	133.6	3.9	43.3
Delay (s)	184.6	170.7	0.2	55.7	186.1	32.2	56.0	68.5	20.1	182.1	32.2	76.4
Level of Service	F	F	A	E	F	C	E	E	C	F	C	E
Approach Delay (s)		147.8			129.6			59.0			69.6	
Approach LOS		F			F			E			E	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			89.8									F
HCM 2000 Volume to Capacity ratio			1.19									
Actuated Cycle Length (s)			114.0						22.0			
Intersection Capacity Utilization			105.3%									G
Analysis Period (min)			15									

c Critical Lane Group

Queues

8: Staples Mill Road & Hungary Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	237	747	617	22	726	641	1369	13	105	722	248
v/c Ratio	0.86	0.60	0.59	0.26	0.99	0.91	0.93	0.02	0.67	0.71	0.39
Control Delay	85.9	42.8	14.1	73.0	86.4	73.5	53.4	0.1	84.3	51.1	7.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	85.9	42.8	14.1	73.0	86.4	73.5	53.4	0.1	84.3	51.1	7.3
Queue Length 50th (ft)	216	319	227	20	~387	303	646	0	97	324	6
Queue Length 95th (ft)	#336	401	362	51	#527	#402	#823	0	160	399	75
Internal Link Dist (ft)		434			495		535			418	
Turn Bay Length (ft)	275		230	200		315		115	340		240
Base Capacity (vph)	310	1236	1053	175	733	724	1467	733	188	1013	629
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.76	0.60	0.59	0.13	0.99	0.89	0.93	0.02	0.56	0.71	0.39

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 8: Staples Mill Road & Hungary Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	220	695	574	20	578	97	596	1273	12	98	671	231
Future Volume (veh/h)	220	695	574	20	578	97	596	1273	12	98	671	231
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1900	1900	1900	1900	1900	1841	1900	1900	1900	1885	1900
Adj Flow Rate, veh/h	237	747	617	22	622	104	641	1369	0	105	722	0
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	1	0	0	0	0	0	4	0	0	0	1	0
Cap, veh/h	261	1078	807	37	537	90	690	1678		128	1191	
Arrive On Green	0.15	0.30	0.30	0.02	0.17	0.17	0.20	0.46	0.00	0.07	0.33	0.00
Sat Flow, veh/h	1795	3610	1610	1810	3095	517	3401	3610	1610	1810	3582	1610
Grp Volume(v), veh/h	237	747	617	22	362	364	641	1369	0	105	722	0
Grp Sat Flow(s),veh/h/ln	1795	1805	1610	1810	1805	1807	1700	1805	1610	1810	1791	1610
Q Serve(g_s), s	18.7	26.4	43.0	1.7	25.0	25.0	26.7	47.1	0.0	8.2	24.3	0.0
Cycle Q Clear(g_c), s	18.7	26.4	43.0	1.7	25.0	25.0	26.7	47.1	0.0	8.2	24.3	0.0
Prop In Lane	1.00		1.00	1.00		0.29	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	261	1078	807	37	313	314	690	1678		128	1191	
V/C Ratio(X)	0.91	0.69	0.76	0.60	1.16	1.16	0.93	0.82		0.82	0.61	
Avail Cap(c_a), veh/h	312	1078	807	176	313	314	732	1678		188	1191	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	60.6	44.7	29.0	70.0	59.5	59.5	56.4	33.2	0.0	66.0	40.2	0.0
Incr Delay (d2), s/veh	25.4	1.9	4.4	11.0	100.0	101.3	17.5	4.5	0.0	14.2	2.3	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	10.4	12.1	18.0	0.9	20.1	20.3	13.1	21.5	0.0	4.3	11.2	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	86.0	46.6	33.4	81.0	159.5	160.8	73.9	37.7	0.0	80.3	42.5	0.0
LnGrp LOS	F	D	C	F	F	F	E	D		F	D	
Approach Vol, veh/h		1601			748			2010	A		827	A
Approach Delay, s/veh		47.3			157.9			49.3			47.3	
Approach LOS		D			F			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	15.2	72.9	7.9	48.0	34.2	53.9	25.9	30.0				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	54.0	14.0	38.0	31.0	40.0	25.0	25.0				
Max Q Clear Time (g_c+l1), s	10.2	49.1	3.7	45.0	28.7	26.3	20.7	27.0				
Green Ext Time (p_c), s	0.1	4.3	0.0	0.0	0.6	6.6	0.2	0.0				

Intersection Summary

HCM 6th Ctrl Delay	64.0
HCM 6th LOS	E

Notes

Unsignalized Delay for [NBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Queues

9: Staples Mill Road & Hungary Spring Road

10/12/2021


























Lane Group	EBL	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	539	302	149	278	60	203	1357	131	49	940	301
v/c Ratio	0.90	0.49	0.26	0.78	0.21	0.83	0.79	0.16	0.39	0.67	0.37
Control Delay	66.8	46.5	10.1	67.0	1.7	75.7	30.7	3.8	59.9	32.2	4.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.8	46.5	10.1	67.0	1.7	75.7	30.7	3.8	59.9	32.2	4.6
Queue Length 50th (ft)	205	108	22	107	0	148	453	0	36	307	4
Queue Length 95th (ft)	#301	154	67	#168	0	#263	582	35	74	383	61
Internal Link Dist (ft)		599		369			448			430	
Turn Bay Length (ft)	340		340		80	210			280		215
Base Capacity (vph)	603	622	591	365	285	264	1725	840	202	1409	813
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.89	0.49	0.25	0.76	0.21	0.77	0.79	0.16	0.24	0.67	0.37

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 9: Staples Mill Road & Hungary Spring Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	512	287	142	88	176	57	193	1289	124	47	893	286
Future Volume (vph)	512	287	142	88	176	57	193	1289	124	47	893	286
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	5.0		6.0	6.0	5.0	6.0	6.0	5.0	6.0	6.0
Lane Util. Factor	0.97	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frpb, ped/bikes	1.00	1.00	0.99		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00		0.98	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3502	3610	1601		3527	1615	1805	3610	1615	1805	3574	1615
Flt Permitted	0.95	1.00	1.00		0.98	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3502	3610	1601		3527	1615	1805	3610	1615	1805	3574	1615
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	539	302	149	93	185	60	203	1357	131	49	940	301
RTOR Reduction (vph)	0	0	71	0	0	54	0	0	70	0	0	177
Lane Group Flow (vph)	539	302	78	0	278	6	203	1357	61	49	940	124
Confl. Peds. (#/hr)			4	4								
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	1%	0%
Turn Type	Split	NA	pm+ov	Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	4	4	5	3	3		5	2		1	6	
Permitted Phases			4			3			2			6
Actuated Green, G (s)	19.8	19.8	35.6		11.7	11.7	15.8	54.4	54.4	7.1	45.7	45.7
Effective Green, g (s)	19.8	19.8	35.6		11.7	11.7	15.8	54.4	54.4	7.1	45.7	45.7
Actuated g/C Ratio	0.17	0.17	0.31		0.10	0.10	0.14	0.47	0.47	0.06	0.39	0.39
Clearance Time (s)	6.0	6.0	5.0		6.0	6.0	5.0	6.0	6.0	5.0	6.0	6.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5	2.5	6.0	6.0	2.5	6.0	6.0
Lane Grp Cap (vph)	597	616	491		355	162	245	1692	757	110	1408	636
v/s Ratio Prot	c0.15	0.08	0.02		c0.08		c0.11	c0.38		0.03	0.26	
v/s Ratio Perm			0.03			0.00			0.04			0.08
v/c Ratio	0.90	0.49	0.16		0.78	0.04	0.83	0.80	0.08	0.45	0.67	0.20
Uniform Delay, d1	47.2	43.5	29.3		50.9	47.1	48.8	26.2	17.0	52.6	28.9	23.1
Progression Factor	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	17.0	0.4	0.1		10.4	0.1	19.7	4.1	0.2	2.1	2.5	0.7
Delay (s)	64.1	44.0	29.4		61.3	47.1	68.5	30.3	17.2	54.6	31.4	23.8
Level of Service	E	D	C		E	D	E	C	B	D	C	C
Approach Delay (s)		52.7			58.8			33.9			30.5	
Approach LOS		D			E			C			C	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			39.2		HCM 2000 Level of Service					D		
HCM 2000 Volume to Capacity ratio			0.85									
Actuated Cycle Length (s)			116.0		Sum of lost time (s)					23.0		
Intersection Capacity Utilization			81.0%		ICU Level of Service					D		
Analysis Period (min)			15									

c Critical Lane Group

Queues

10: Staples Mill Road & Staples Mill Square Shopping Center

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	76	77	288	80	82	188	312	1329	183	173	859	171
v/c Ratio	0.50	0.49	0.59	0.63	0.62	0.64	0.91	0.76	0.21	0.95	0.49	0.19
Control Delay	61.9	61.3	27.6	73.5	71.5	17.8	83.6	28.1	2.8	107.1	21.3	2.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.9	61.3	27.6	73.5	71.5	17.8	83.6	28.1	2.8	107.1	21.3	2.3
Queue Length 50th (ft)	57	57	110	61	63	0	122	415	0	~146	222	0
Queue Length 95th (ft)	110	111	206	#117	118	71	#216	504	34	#289	279	29
Internal Link Dist (ft)		193			125			406			259	
Turn Bay Length (ft)	250		130	175		175	350			215		615
Base Capacity (vph)	162	166	478	144	151	308	342	1745	868	183	1748	886
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.47	0.46	0.60	0.56	0.54	0.61	0.91	0.76	0.21	0.95	0.49	0.19

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

























# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 10: Staples Mill Road & Staples Mill Square Shopping Center

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	103	40	271	103	49	177	293	1249	172	163	807	161
Future Volume (vph)	103	40	271	103	49	177	293	1249	172	163	807	161
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.3	7.3	7.3	8.0	8.0	8.0	7.6	5.9	5.9	7.8	5.9	5.9
Lane Util. Factor	0.95	0.95	1.00	0.95	0.95	1.00	0.97	0.95	1.00	1.00	0.95	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.99	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	0.98	1.00	0.95	0.98	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1715	1766	1583	1681	1760	1589	3502	3610	1595	1805	3574	1615
Flt Permitted	0.95	0.98	1.00	0.95	0.98	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1715	1766	1583	1681	1760	1589	3502	3610	1595	1805	3574	1615
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	110	43	288	110	52	188	312	1329	183	173	859	171
RTOR Reduction (vph)	0	0	89	0	0	174	0	0	94	0	0	87
Lane Group Flow (vph)	76	77	199	80	82	14	312	1329	89	173	859	84
Confl. Peds. (#/hr)	1					1			1	1		
Heavy Vehicles (%)	0%	0%	2%	2%	0%	0%	0%	0%	0%	0%	1%	0%
Turn Type	Split	NA	pt+ov	Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	4	4	4 5	3	3		5	2		1	6	
Permitted Phases						3			2			6
Actuated Green, G (s)	10.3	10.3	28.9	8.8	8.8	8.8	11.3	56.1	56.1	11.8	56.8	56.8
Effective Green, g (s)	10.3	10.3	28.9	8.8	8.8	8.8	11.3	56.1	56.1	11.8	56.8	56.8
Actuated g/C Ratio	0.09	0.09	0.25	0.08	0.08	0.08	0.10	0.48	0.48	0.10	0.49	0.49
Clearance Time (s)	7.3	7.3		8.0	8.0	8.0	7.6	5.9	5.9	7.8	5.9	5.9
Vehicle Extension (s)	2.0	2.0		2.0	2.0	2.0	2.0	6.0	6.0	2.0	6.0	6.0
Lane Grp Cap (vph)	152	156	394	127	133	120	341	1745	771	183	1750	790
v/s Ratio Prot	0.04	0.04	c0.13	c0.05	0.05		0.09	c0.37		c0.10	0.24	
v/s Ratio Perm						0.01			0.06			0.05
v/c Ratio	0.50	0.49	0.50	0.63	0.62	0.12	0.91	0.76	0.11	0.95	0.49	0.11
Uniform Delay, d1	50.4	50.4	37.4	52.0	52.0	50.0	51.9	24.5	16.4	51.8	19.9	15.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.9	0.9	0.4	6.9	5.9	0.2	27.6	3.2	0.3	50.0	1.0	0.3
Delay (s)	51.3	51.3	37.8	58.9	57.8	50.1	79.5	27.7	16.7	101.8	20.9	16.2
Level of Service	D	D	D	E	E	D	E	C	B	F	C	B
Approach Delay (s)		42.5			53.9			35.4			31.8	
Approach LOS		D			D			D			C	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			36.8				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			0.77									
Actuated Cycle Length (s)			116.0				Sum of lost time (s)			29.0		
Intersection Capacity Utilization			72.4%				ICU Level of Service			C		
Analysis Period (min)			15									

c Critical Lane Group



Queues

11: Staples Mill Road & Old Staples Mill Road / Lucas Road

10/12/2021



Lane Group	EBT	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	84	250	88	101	1679	213	105	1151	3
v/c Ratio	0.48	0.79	0.21	0.58	0.92	0.23	0.69	0.66	0.00
Control Delay	33.0	63.7	1.1	62.8	38.6	3.2	74.4	27.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.0	63.7	1.1	62.8	38.6	3.2	74.4	27.0	0.0
Queue Length 50th (ft)	25	176	0	73	~703	0	76	353	0
Queue Length 95th (ft)	73	#303	0	126	#852	43	#158	486	0
Internal Link Dist (ft)	151	218			318			355	
Turn Bay Length (ft)			50	150		450	250		200
Base Capacity (vph)	254	327	433	241	1816	918	161	1747	871
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.76	0.20	0.42	0.92	0.23	0.65	0.66	0.00

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 11: Staples Mill Road & Old Staples Mill Road / Lucas Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕	↕	↕	↕	↕	↕	↕
Traffic Volume (vph)	1	31	47	209	26	83	95	1578	200	99	1082	3
Future Volume (vph)	1	31	47	209	26	83	95	1578	200	99	1082	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Lane Util. Factor		1.00			1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.92			1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		1.00			0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1746			1793	1615	1752	3610	1615	1805	3574	1615
Flt Permitted		1.00			0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1746			1793	1615	1752	3610	1615	1805	3574	1615
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	1	33	50	222	28	88	101	1679	213	105	1151	3
RTOR Reduction (vph)	0	47	0	0	0	72	0	0	108	0	0	2
Lane Group Flow (vph)	0	37	0	0	250	16	101	1679	105	105	1151	1
Heavy Vehicles (%)	0%	0%	0%	1%	5%	0%	3%	0%	0%	0%	1%	0%
Turn Type	Split	NA		Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	3	3		4	4		5	2		1	6	
Permitted Phases						4			2			6
Actuated Green, G (s)		7.3			20.5	20.5	11.5	57.4	57.4	9.8	55.7	55.7
Effective Green, g (s)		7.3			20.5	20.5	11.5	57.4	57.4	9.8	55.7	55.7
Actuated g/C Ratio		0.06			0.18	0.18	0.10	0.49	0.49	0.08	0.48	0.48
Clearance Time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0
Vehicle Extension (s)		3.0			3.0	3.0	2.5	4.5	4.5	2.5	4.5	4.5
Lane Grp Cap (vph)		109			316	285	173	1786	799	152	1716	775
v/s Ratio Prot		c0.02			c0.14		0.06	c0.47		c0.06	0.32	
v/s Ratio Perm						0.01			0.07			0.00
v/c Ratio		0.34			0.79	0.05	0.58	0.94	0.13	0.69	0.67	0.00
Uniform Delay, d1		52.0			45.7	39.7	50.0	27.7	15.8	51.6	23.1	15.7
Progression Factor		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		1.9			12.7	0.1	4.1	11.2	0.3	11.8	2.1	0.0
Delay (s)		53.9			58.4	39.8	54.1	38.9	16.2	63.4	25.2	15.7
Level of Service		D			E	D	D	D	B	E	C	B
Approach Delay (s)		53.9			53.5			37.2			28.4	
Approach LOS		D			D			D			C	

### Intersection Summary

HCM 2000 Control Delay	36.1	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.84		
Actuated Cycle Length (s)	116.0	Sum of lost time (s)	21.0
Intersection Capacity Utilization	82.4%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			

Queues

12: Staples Mill Road & E Parham Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	166	934	304	376	682	246	255	1463	385	277	1142
v/c Ratio	0.64	1.04	0.44	1.05	0.67	0.34	0.69	1.15	0.49	0.79	0.65
Control Delay	64.0	82.7	20.1	111.5	40.9	16.4	59.6	111.9	17.1	67.3	33.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	82.7	20.1	111.5	40.9	16.4	59.6	111.9	17.1	67.3	33.7
Queue Length 50th (ft)	62	~396	118	~157	238	81	95	~681	143	105	261
Queue Length 95th (ft)	100	#526	193	#256	305	144	138	#820	223	#164	315
Internal Link Dist (ft)		473			499			459			344
Turn Bay Length (ft)	280		470	200		390	320			240	
Base Capacity (vph)	268	900	712	358	1011	735	414	1273	779	362	1761
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.62	1.04	0.43	1.05	0.67	0.33	0.62	1.15	0.49	0.77	0.65

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

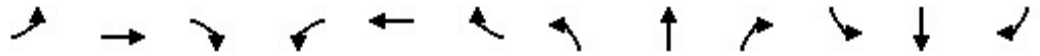
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 12: Staples Mill Road & E Parham Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗	↑↑	↖	↖↗	↑↑	↖	↖↗	↑↑	↖	↖↗	↑↑↗	
Traffic Volume (veh/h)	161	906	295	365	662	239	247	1419	373	269	1020	87
Future Volume (veh/h)	161	906	295	365	662	239	247	1419	373	269	1020	87
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1870	1870	1885	1885	1885	1870	1885	1885	1900	1870	1870
Adj Flow Rate, veh/h	166	934	0	376	682	246	255	1463	0	277	1052	90
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	1	2	2	1	1	1	2	1	1	0	2	2
Cap, veh/h	224	904		360	1051	621	317	1295		335	1750	150
Arrive On Green	0.06	0.25	0.00	0.10	0.29	0.29	0.09	0.36	0.00	0.10	0.37	0.37
Sat Flow, veh/h	3483	3554	1585	3483	3582	1598	3456	3582	1598	3510	4791	409
Grp Volume(v), veh/h	166	934	0	376	682	246	255	1463	0	277	747	395
Grp Sat Flow(s),veh/h/ln	1742	1777	1585	1742	1791	1598	1728	1791	1598	1755	1702	1796
Q Serve(g_s), s	5.4	29.5	0.0	12.0	19.3	12.9	8.4	41.9	0.0	9.0	20.7	20.8
Cycle Q Clear(g_c), s	5.4	29.5	0.0	12.0	19.3	12.9	8.4	41.9	0.0	9.0	20.7	20.8
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.23
Lane Grp Cap(c), veh/h	224	904		360	1051	621	317	1295		335	1243	656
V/C Ratio(X)	0.74	1.03		1.04	0.65	0.40	0.81	1.13		0.83	0.60	0.60
Avail Cap(c_a), veh/h	270	904		360	1051	621	417	1295		363	1243	656
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.3	43.3	0.0	52.0	35.8	25.6	51.7	37.0	0.0	51.5	29.9	30.0
Incr Delay (d2), s/veh	7.6	38.9	0.0	59.2	3.1	1.9	7.5	68.8	0.0	13.2	2.2	4.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.6	17.7	0.0	8.2	8.8	5.2	4.0	30.3	0.0	4.6	8.8	9.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.9	82.1	0.0	111.2	38.9	27.5	59.2	105.8	0.0	64.7	32.1	34.0
LnGrp LOS	E	F		F	D	C	E	F		E	C	C
Approach Vol, veh/h		1100	A		1304			1718	A		1419	
Approach Delay, s/veh		78.9			57.6			98.9			39.0	
Approach LOS		E			E			F			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	16.1	47.9	12.5	39.5	15.6	48.4	17.0	35.0				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.5	5.0	6.0	5.0	5.5				
Max Green Setting (Gmax), s	12.0	41.0	9.0	32.5	14.0	39.0	12.0	29.5				
Max Q Clear Time (g_c+I1), s	11.0	43.9	7.4	21.3	10.4	22.8	14.0	31.5				
Green Ext Time (p_c), s	0.1	0.0	0.1	8.3	0.2	14.0	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay	69.9
HCM 6th LOS	E

Notes

Unsignalized Delay for [NBR, EBR] is excluded from calculations of the approach delay and intersection delay.

Queues

13: Staples Mill Road & Hermitage Road / Commercial Ent.

10/12/2021
























Lane Group	EBT	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	33	198	156	39	1913	174	184	1681
v/c Ratio	0.31	0.81	0.31	0.38	0.71	0.19	0.82	0.51
Control Delay	51.6	74.0	12.8	63.2	23.3	3.0	77.6	13.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	51.6	74.0	12.8	63.2	23.3	3.0	77.6	13.7
Queue Length 50th (ft)	19	144	32	29	426	0	135	292
Queue Length 95th (ft)	52	#255	76	65	491	37	#250	338
Internal Link Dist (ft)	192	548			450			520
Turn Bay Length (ft)			225	315		395	165	
Base Capacity (vph)	114	260	517	107	2688	932	234	3302
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.29	0.76	0.30	0.36	0.71	0.19	0.79	0.51

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 13: Staples Mill Road & Hermitage Road / Commercial Ent.

10/12/2021

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	19	6	7	183	9	151	38	1856	169	178	1607	23	
Future Volume (vph)	19	6	7	183	9	151	38	1856	169	178	1607	23	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0		
Lane Util. Factor		1.00			1.00	1.00	1.00	0.91	1.00	1.00	0.91		
Frbp, ped/bikes		1.00			1.00	1.00	1.00	1.00	0.98	1.00	1.00		
Flpb, ped/bikes		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Frt		0.97			1.00	0.85	1.00	1.00	0.85	1.00	1.00		
Flt Protected		0.97			0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (prot)		1791			1779	1615	1787	4988	1581	1805	5124		
Flt Permitted		0.97			0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (perm)		1791			1779	1615	1787	4988	1581	1805	5124		
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	20	6	7	189	9	156	39	1913	174	184	1657	24	
RTOR Reduction (vph)	0	7	0	0	0	61	0	0	83	0	1	0	
Lane Group Flow (vph)	0	26	0	0	198	95	39	1913	91	184	1680	0	
Confl. Peds. (#/hr)							2		1	1		2	
Heavy Vehicles (%)	0%	0%	0%	2%	0%	0%	1%	4%	0%	0%	1%	0%	
Turn Type	Split	NA		Split	NA	pm+ov	Prot	NA	Perm	Prot	NA		
Protected Phases	4	4		3	3	1	5	2		1	6		
Permitted Phases						3			2				
Actuated Green, G (s)		4.2			15.9	30.3	4.2	60.5	60.5	14.4	70.7		
Effective Green, g (s)		4.2			15.9	30.3	4.2	60.5	60.5	14.4	70.7		
Actuated g/C Ratio		0.04			0.14	0.26	0.04	0.52	0.52	0.12	0.61		
Clearance Time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0		
Vehicle Extension (s)		3.0			3.0	3.0	3.0	5.0	5.0	3.0	5.0		
Lane Grp Cap (vph)		64			243	421	64	2601	824	224	3122		
v/s Ratio Prot		c0.01			c0.11	0.03	0.02	c0.38		c0.10	0.33		
v/s Ratio Perm						0.03			0.06				
v/c Ratio		0.41			0.81	0.22	0.61	0.74	0.11	0.82	0.54		
Uniform Delay, d1		54.7			48.6	33.6	55.1	21.5	14.1	49.5	13.2		
Progression Factor		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2		4.2			18.6	0.3	15.3	1.9	0.3	20.9	0.7		
Delay (s)		58.9			67.2	33.9	70.4	23.4	14.4	70.4	13.8		
Level of Service		E			E	C	E	C	B	E	B		
Approach Delay (s)		58.9			52.5			23.6			19.4		
Approach LOS		E			D			C			B		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			24.4		HCM 2000 Level of Service					C			
HCM 2000 Volume to Capacity ratio			0.75										
Actuated Cycle Length (s)			116.0		Sum of lost time (s)					21.0			
Intersection Capacity Utilization			74.7%		ICU Level of Service					D			
Analysis Period (min)			15										

c Critical Lane Group

Queues

14: Staples Mill Road & Wistar Road

10/12/2021



























Lane Group	EBT	EBR	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	183	204	77	187	1975	18	1759	185
v/c Ratio	0.87	0.53	0.45	0.82	0.56	0.17	0.61	0.19
Control Delay	83.1	17.3	41.0	77.3	10.7	54.9	18.1	2.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	83.1	17.3	41.0	77.3	10.7	54.9	18.1	2.3
Queue Length 50th (ft)	134	29	37	137	205	13	313	0
Queue Length 95th (ft)	#257	103	87	#252	368	37	361	32
Internal Link Dist (ft)	312		290		483		269	
Turn Bay Length (ft)		215		415		275		1000
Base Capacity (vph)	221	394	179	238	3519	122	2901	984
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.52	0.43	0.79	0.56	0.15	0.61	0.19

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 14: Staples Mill Road & Wistar Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations								  			  	
Traffic Volume (vph)	170	8	198	41	6	28	181	1893	22	17	1706	179
Future Volume (vph)	170	8	198	41	6	28	181	1893	22	17	1706	179
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0		6.0		5.5	6.0		5.5	6.0	6.0
Lane Util. Factor		1.00	1.00		1.00		1.00	0.91		1.00	0.91	1.00
Frbp, ped/bikes		1.00	1.00		1.00		1.00	1.00		1.00	1.00	1.00
Flpb, ped/bikes		1.00	1.00		1.00		1.00	1.00		1.00	1.00	1.00
Frt		1.00	0.85		0.95		1.00	1.00		1.00	1.00	0.85
Flt Protected		0.95	1.00		0.97		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)		1813	1599		1729		1787	5076		1671	5136	1599
Flt Permitted		0.71	1.00		0.55		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)		1350	1599		981		1787	5076		1671	5136	1599
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	175	8	204	42	6	29	187	1952	23	18	1759	185
RTOR Reduction (vph)	0	0	134	0	19	0	0	1	0	0	0	81
Lane Group Flow (vph)	0	183	70	0	58	0	187	1974	0	18	1759	104
Confl. Peds. (#/hr)									1	1		
Heavy Vehicles (%)	0%	0%	1%	0%	0%	4%	1%	2%	0%	8%	1%	1%
Turn Type	Perm	NA	Perm	Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4			4		5	2		1	6	
Permitted Phases	4		4	4								6
Actuated Green, G (s)		18.2	18.2		18.2		14.8	77.1		3.2	65.5	65.5
Effective Green, g (s)		18.2	18.2		18.2		14.8	77.1		3.2	65.5	65.5
Actuated g/C Ratio		0.16	0.16		0.16		0.13	0.66		0.03	0.56	0.56
Clearance Time (s)		6.0	6.0		6.0		5.5	6.0		5.5	6.0	6.0
Vehicle Extension (s)		3.5	3.5		3.5		3.0	5.0		3.0	5.0	5.0
Lane Grp Cap (vph)		211	250		153		227	3373		46	2900	902
v/s Ratio Prot							c0.10	c0.39		0.01	0.34	
v/s Ratio Perm		c0.14	0.04		0.06							0.07
v/c Ratio		0.87	0.28		0.38		0.82	0.59		0.39	0.61	0.12
Uniform Delay, d1		47.7	43.1		43.9		49.3	10.7		55.4	16.7	11.8
Progression Factor		1.00	1.00		1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2		29.7	0.7		1.9		20.9	0.7		5.4	1.0	0.3
Delay (s)		77.4	43.8		45.7		70.2	11.4		60.9	17.7	12.0
Level of Service		E	D		D		E	B		E	B	B
Approach Delay (s)		59.7			45.7		16.5			17.5		
Approach LOS		E			D		B			B		
<b>Intersection Summary</b>												
HCM 2000 Control Delay			21.1				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.69									
Actuated Cycle Length (s)			116.0				Sum of lost time (s)			17.5		
Intersection Capacity Utilization			73.2%				ICU Level of Service			D		
Analysis Period (min)			15									

c Critical Lane Group



Queues

15: Staples Mill Road & Bremner Boulevard

10/12/2021



Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	110	125	64	25	131	2143	45	1993
v/c Ratio	0.72	0.50	0.48	0.11	0.76	0.67	0.33	0.67
Control Delay	78.1	16.3	62.9	1.0	77.9	17.4	57.3	19.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	78.1	16.3	62.9	1.0	77.9	17.4	57.3	19.2
Queue Length 50th (ft)	81	0	46	0	96	402	33	389
Queue Length 95th (ft)	#169	58	92	0	#186	490	69	447
Internal Link Dist (ft)	178		254			316		213
Turn Bay Length (ft)		115		100	315		320	
Base Capacity (vph)	157	253	155	244	184	3198	171	2959
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.70	0.49	0.41	0.10	0.71	0.67	0.26	0.67

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 15: Staples Mill Road & Bremner Boulevard

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖	↗		↖	↗	↖	↑↑↑		↖	↑↑↑	
Traffic Volume (vph)	92	15	121	48	15	24	127	2058	20	44	1848	85
Future Volume (vph)	92	15	121	48	15	24	127	2058	20	44	1848	85
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0	
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	0.91		1.00	0.91	
Frbp, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	1.00		1.00	0.99	
Flt Protected		0.96	1.00		0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1775	1568		1802	1538	1787	5121		1805	5095	
Flt Permitted		0.96	1.00		0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)		1775	1568		1802	1538	1787	5121		1805	5095	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	95	15	125	49	15	25	131	2122	21	45	1905	88
RTOR Reduction (vph)	0	0	114	0	0	23	0	1	0	0	4	0
Lane Group Flow (vph)	0	110	11	0	64	2	131	2142	0	45	1989	0
Confl. Peds. (#/hr)							1		1	1		1
Heavy Vehicles (%)	3%	0%	3%	2%	0%	5%	1%	1%	13%	0%	1%	2%
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases			4			3						
Actuated Green, G (s)		9.9	9.9		7.5	7.5	11.2	70.4		7.2	66.4	
Effective Green, g (s)		9.9	9.9		7.5	7.5	11.2	70.4		7.2	66.4	
Actuated g/C Ratio		0.09	0.09		0.06	0.06	0.10	0.61		0.06	0.57	
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0	
Vehicle Extension (s)		3.0	3.0		3.0	3.0	2.5	6.0		2.5	6.0	
Lane Grp Cap (vph)		151	133		116	99	172	3107		112	2916	
v/s Ratio Prot		c0.06			c0.04		c0.07	c0.42		0.02	0.39	
v/s Ratio Perm			0.01			0.00						
v/c Ratio		0.73	0.08		0.55	0.02	0.76	0.69		0.40	0.68	
Uniform Delay, d1		51.7	48.9		52.6	50.8	51.1	15.4		52.3	17.4	
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2		16.1	0.3		5.6	0.1	17.2	1.3		1.7	1.3	
Delay (s)		67.8	49.1		58.2	50.9	68.3	16.7		54.0	18.7	
Level of Service		E	D		E	D	E	B		D	B	
Approach Delay (s)		57.9			56.1			19.7			19.5	
Approach LOS		E			E			B			B	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			22.2				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.71									
Actuated Cycle Length (s)			116.0				Sum of lost time (s)			21.0		
Intersection Capacity Utilization			72.8%				ICU Level of Service			C		
Analysis Period (min)			15									
c Critical Lane Group												

Queues

16: Staples Mill Road & Amtrak Station

10/12/2021




















Lane Group	WBL	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	11	2	2284	5	2095
v/c Ratio	0.10	0.02	0.49	0.06	0.44
Control Delay	53.0	36.0	3.4	53.0	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	53.0	36.0	3.4	53.0	1.7
Queue Length 50th (ft)	8	0	0	4	0
Queue Length 95th (ft)	27	9	336	17	147
Internal Link Dist (ft)	469		187		179
Turn Bay Length (ft)				170	
Base Capacity (vph)	290	262	4650	151	4783
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.04	0.01	0.49	0.03	0.44

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 16: Staples Mill Road & Amtrak Station

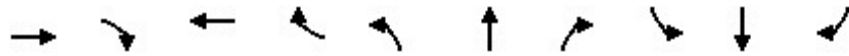
10/12/2021

							
Movement	WBL	WBR	NBU	NBT	NBR	SBL	SBT
Lane Configurations				  			  
Traffic Volume (vph)	11	2	0	2206	10	5	2032
Future Volume (vph)	11	2	0	2206	10	5	2032
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3		5.9		6.8	5.9
Lane Util. Factor	1.00	1.00		0.91		1.00	0.91
Frbp, ped/bikes	1.00	1.00		1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00		1.00		1.00	1.00
Frt	1.00	0.85		1.00		1.00	1.00
Flt Protected	0.95	1.00		1.00		0.95	1.00
Satd. Flow (prot)	1805	1615		5126		1444	5136
Flt Permitted	0.95	1.00		1.00		0.95	1.00
Satd. Flow (perm)	1805	1615		5126		1444	5136
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	11	2	0	2274	10	5	2095
RTOR Reduction (vph)	0	2	0	0	0	0	0
Lane Group Flow (vph)	11	0	0	2284	0	5	2095
Confl. Peds. (#/hr)					1	1	
Heavy Vehicles (%)	0%	0%	0%	1%	25%	25%	1%
Turn Type	Prot	Perm	Prot	NA		Prot	NA
Protected Phases	4		5	2		1	6
Permitted Phases		4					
Actuated Green, G (s)	3.1	3.1		92.5		1.4	100.7
Effective Green, g (s)	3.1	3.1		92.5		1.4	100.7
Actuated g/C Ratio	0.03	0.03		0.80		0.01	0.87
Clearance Time (s)	6.3	6.3		5.9		6.8	5.9
Vehicle Extension (s)	3.5	3.5		5.5		3.0	5.5
Lane Grp Cap (vph)	48	43		4087		17	4458
v/s Ratio Prot	c0.01			c0.45		0.00	c0.41
v/s Ratio Perm		0.00					
v/c Ratio	0.23	0.00		0.56		0.29	0.47
Uniform Delay, d1	55.3	54.9		4.3		56.8	1.7
Progression Factor	1.00	1.00		1.00		1.00	1.00
Incremental Delay, d2	2.9	0.0		0.6		9.4	0.4
Delay (s)	58.2	55.0		4.8		66.2	2.1
Level of Service	E	D		A		E	A
Approach Delay (s)	57.7			4.8			2.2
Approach LOS	E			A			A
<b>Intersection Summary</b>							
HCM 2000 Control Delay			3.7		HCM 2000 Level of Service		A
HCM 2000 Volume to Capacity ratio			0.57				
Actuated Cycle Length (s)			116.0		Sum of lost time (s)		19.0
Intersection Capacity Utilization			58.8%		ICU Level of Service		B
Analysis Period (min)			15				
c Critical Lane Group							

Queues

17: Staples Mill Road & Crockett Street

10/12/2021



Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	165	61	60	28	81	2118	20	39	1998	76
v/c Ratio	0.72	0.20	0.44	0.12	0.52	0.68	0.02	0.34	0.69	0.08
Control Delay	66.3	1.4	60.6	1.1	58.2	7.6	0.0	58.9	22.7	1.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.3	1.4	60.6	1.1	58.2	7.6	0.0	58.9	22.7	1.4
Queue Length 50th (ft)	119	0	44	0	63	150	0	28	409	0
Queue Length 95th (ft)	190	0	86	0	m62	m142	m0	63	562	11
Internal Link Dist (ft)	284		261			193			145	
Turn Bay Length (ft)		115			190		75	295		105
Base Capacity (vph)	278	348	203	286	205	3115	948	233	2912	964
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.59	0.18	0.30	0.10	0.40	0.68	0.02	0.17	0.69	0.08























Intersection Summary

m Volume for 95th percentile queue is metered by upstream signal.

# HCM Signalized Intersection Capacity Analysis

## 17: Staples Mill Road & Crockett Street

10/12/2021

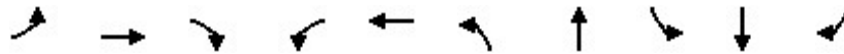
														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations														
Traffic Volume (vph)	153	7	59	52	6	27	79	2054	19	38	1938	74		
Future Volume (vph)	153	7	59	52	6	27	79	2054	19	38	1938	74		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0		
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	0.91	1.00	1.00	0.91	1.00		
Frbp, ped/bikes		1.00	1.00		1.00	0.98	1.00	1.00	0.99	1.00	1.00	1.00		
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Frt		1.00	0.85		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85		
Flt Protected		0.95	1.00		0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00		
Satd. Flow (prot)		1796	1583		1818	1588	1805	5136	1490	1805	5136	1615		
Flt Permitted		0.95	1.00		0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00		
Satd. Flow (perm)		1796	1583		1818	1588	1805	5136	1490	1805	5136	1615		
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97		
Adj. Flow (vph)	158	7	61	54	6	28	81	2118	20	39	1998	76		
RTOR Reduction (vph)	0	0	53	0	0	26	0	0	8	0	0	34		
Lane Group Flow (vph)	0	165	8	0	60	2	81	2118	12	39	1998	42		
Confl. Peds. (#/hr)	1					1			1	1				
Heavy Vehicles (%)	1%	0%	2%	0%	0%	0%	0%	1%	7%	0%	1%	0%		
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm		
Protected Phases	3	3		4	4		5	2		1	6			
Permitted Phases			3			4			2			6		
Actuated Green, G (s)		14.8	14.8		7.6	7.6	8.8	67.4	67.4	5.2	63.8	63.8		
Effective Green, g (s)		14.8	14.8		7.6	7.6	8.8	67.4	67.4	5.2	63.8	63.8		
Actuated g/C Ratio		0.13	0.13		0.07	0.07	0.08	0.58	0.58	0.04	0.55	0.55		
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0		
Vehicle Extension (s)		2.5	2.5		2.5	2.5	2.5	4.0	4.0	2.5	4.0	4.0		
Lane Grp Cap (vph)		229	201		119	104	136	2984	865	80	2824	888		
v/s Ratio Prot		c0.09			c0.03		c0.04	c0.41		0.02	0.39			
v/s Ratio Perm			0.00			0.00			0.01			0.03		
v/c Ratio		0.72	0.04		0.50	0.02	0.60	0.71	0.01	0.49	0.71	0.05		
Uniform Delay, d1		48.6	44.4		52.4	50.7	51.9	17.3	10.3	54.1	19.2	12.1		
Progression Factor		1.00	1.00		1.00	1.00	1.14	0.40	1.00	1.00	1.00	1.00		
Incremental Delay, d2		10.0	0.1		2.4	0.0	0.5	0.1	0.0	3.4	1.5	0.1		
Delay (s)		58.6	44.4		54.8	50.8	59.8	7.0	10.3	57.5	20.8	12.2		
Level of Service		E	D		D	D	E	A	B	E	C	B		
Approach Delay (s)		54.8			53.5			8.9		21.1				
Approach LOS		D			D			A		C				
<b>Intersection Summary</b>														
HCM 2000 Control Delay			17.6									HCM 2000 Level of Service	B	
HCM 2000 Volume to Capacity ratio			0.70											
Actuated Cycle Length (s)			116.0								21.0			
Intersection Capacity Utilization			72.7%										ICU Level of Service	C
Analysis Period (min)			15											

c Critical Lane Group

Queues

18: Staples Mill Road & Hilliard Road / Glenside Drive

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	242	623	132	217	685	164	1965	324	1563	214
v/c Ratio	0.93	1.03	0.35	0.89	1.10	0.75	1.11	1.04	0.75	0.28
Control Delay	90.5	93.6	10.0	85.6	104.4	70.1	93.9	107.8	34.5	12.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	90.5	93.6	10.0	85.6	104.4	70.1	93.9	107.8	34.5	12.6
Queue Length 50th (ft)	181	~267	0	161	~271	118	~610	~270	222	25
Queue Length 95th (ft)	#334	#386	54	#300	#394	#206	#708	#454	371	m121
Internal Link Dist (ft)		762			867		276		418	
Turn Bay Length (ft)	200		215	725		280		330		245
Base Capacity (vph)	264	602	378	248	624	241	1770	311	2075	769
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.92	1.03	0.35	0.88	1.10	0.68	1.11	1.04	0.75	0.28

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

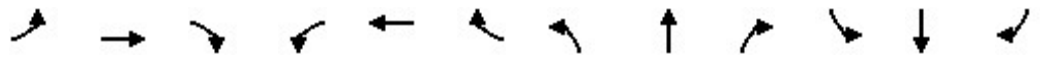
# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

HCM 6th Signalized Intersection Summary  
 18: Staples Mill Road & Hilliard Road / Glenside Drive

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	242	623	132	217	417	268	164	1649	316	324	1563	214
Future Volume (veh/h)	242	623	132	217	417	268	164	1649	316	324	1563	214
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	242	623	132	217	417	268	164	1649	316	324	1563	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	265	591	264	245	322	205	193	1521	289	312	2166	
Arrive On Green	0.15	0.16	0.16	0.14	0.15	0.15	0.11	0.35	0.35	0.17	0.42	0.00
Sat Flow, veh/h	1810	3610	1610	1810	2116	1347	1810	4376	832	1810	5187	1610
Grp Volume(v), veh/h	242	623	132	217	355	330	164	1300	665	324	1563	0
Grp Sat Flow(s),veh/h/ln	1810	1805	1610	1810	1805	1658	1810	1729	1750	1810	1729	1610
Q Serve(g_s), s	15.3	19.0	8.7	13.7	17.7	17.7	10.3	40.3	40.3	20.0	29.1	0.0
Cycle Q Clear(g_c), s	15.3	19.0	8.7	13.7	17.7	17.7	10.3	40.3	40.3	20.0	29.1	0.0
Prop In Lane	1.00		1.00	1.00		0.81	1.00		0.48	1.00		1.00
Lane Grp Cap(c), veh/h	265	591	264	245	275	253	193	1202	608	312	2166	
V/C Ratio(X)	0.91	1.05	0.50	0.89	1.29	1.31	0.85	1.08	1.09	1.04	0.72	
Avail Cap(c_a), veh/h	265	591	264	250	275	253	242	1202	608	312	2166	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	48.8	48.5	44.2	49.3	49.2	49.2	50.9	37.8	37.8	48.0	28.2	0.0
Incr Delay (d2), s/veh	32.5	51.9	1.5	29.2	155.5	163.2	20.0	51.1	64.2	61.3	2.1	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	9.3	12.8	3.6	8.1	19.8	18.7	5.7	25.2	27.7	14.3	12.3	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	81.3	100.4	45.7	78.5	204.6	212.4	70.9	89.0	102.0	109.3	30.3	0.0
LnGrp LOS	F	F	D	E	F	F	E	F	F	F	C	
Approach Vol, veh/h		997			902			2129			1887	A
Approach Delay, s/veh		88.5			177.1			91.6			43.8	
Approach LOS		F			F			F			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	25.0	46.3	20.7	24.0	16.9	54.4	22.0	22.7				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	4.5	6.0	5.0	5.0				
Max Green Setting (Gmax), s	20.0	40.0	16.0	19.0	15.5	45.0	17.0	17.0				
Max Q Clear Time (g_c+I1), s	22.0	42.3	15.7	21.0	12.3	31.1	17.3	19.7				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.0	0.1	10.6	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay	88.9
HCM 6th LOS	F

Notes

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.



Queues

19: Staples Mill Road & Aspen Avenue / Townhouse Road

10/12/2021



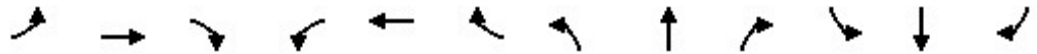
Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	112	139	66	2338	36	2060
v/c Ratio	0.74	0.76	0.41	0.71	0.23	0.65
Control Delay	68.8	73.9	17.4	16.9	9.6	16.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	68.8	73.9	17.4	16.9	9.6	16.6
Queue Length 50th (ft)	64	96	14	451	8	367
Queue Length 95th (ft)	#151	#191	43	524	18	427
Internal Link Dist (ft)	218	764		385		854
Turn Bay Length (ft)			230		215	
Base Capacity (vph)	158	192	174	3273	174	3156
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.71	0.72	0.38	0.71	0.21	0.65

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 19: Staples Mill Road & Aspen Avenue / Townhouse Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↑↑↑		↕	↑↑↑	
Traffic Volume (vph)	49	10	44	91	13	24	61	2101	50	33	1837	58
Future Volume (vph)	49	10	44	91	13	24	61	2101	50	33	1837	58
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0		5.0	6.0		5.0	6.0	
Lane Util. Factor		1.00			1.00		1.00	0.91		1.00	0.91	
Frt		0.94			0.97		1.00	1.00		1.00	1.00	
Flt Protected		0.98			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1749			1788		1805	5169		1805	5163	
Flt Permitted		0.98			0.97		0.06	1.00		0.06	1.00	
Satd. Flow (perm)		1749			1788		106	5169		109	5163	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	53	11	48	99	14	26	66	2284	54	36	1997	63
RTOR Reduction (vph)	0	23	0	0	7	0	0	2	0	0	3	0
Lane Group Flow (vph)	0	89	0	0	132	0	66	2336	0	36	2057	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Split	NA		Split	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases							2			6		
Actuated Green, G (s)		8.5			11.4		76.6	71.4		73.6	69.9	
Effective Green, g (s)		8.5			11.4		76.6	71.4		73.6	69.9	
Actuated g/C Ratio		0.07			0.10		0.66	0.62		0.63	0.60	
Clearance Time (s)		5.0			5.0		5.0	6.0		5.0	6.0	
Vehicle Extension (s)		3.0			3.0		2.5	4.0		2.5	4.0	
Lane Grp Cap (vph)		128			175		146	3181		123	3111	
v/s Ratio Prot		c0.05			c0.07		c0.02	c0.45		0.01	0.40	
v/s Ratio Perm							0.28			0.18		
v/c Ratio		0.69			0.75		0.45	0.73		0.29	0.66	
Uniform Delay, d1		52.5			50.9		12.6	15.6		13.6	15.2	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		15.1			16.6		1.6	1.5		1.0	1.1	
Delay (s)		67.5			67.6		14.2	17.2		14.6	16.3	
Level of Service		E			E		B	B		B	B	
Approach Delay (s)		67.5			67.6		17.1			16.3		
Approach LOS		E			E		B			B		
<b>Intersection Summary</b>												
HCM 2000 Control Delay			19.4				HCM 2000 Level of Service				B	
HCM 2000 Volume to Capacity ratio			0.73									
Actuated Cycle Length (s)			116.0				Sum of lost time (s)			21.0		
Intersection Capacity Utilization			70.1%				ICU Level of Service				C	
Analysis Period (min)			15									
c Critical Lane Group												

Queues

20: Staples Mill Road & Dumbarton Road / Wharfside Road

10/12/2021



Lane Group	EBL	EBT	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	7	11	441	286	2147	1016	380	1813
v/c Ratio	0.09	0.06	0.82	0.39	0.94	1.00	0.90	0.49
Control Delay	56.0	0.6	61.4	18.6	40.7	43.9	69.5	7.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.0	0.6	61.4	18.6	40.7	43.9	69.5	7.9
Queue Length 50th (ft)	5	0	160	93	550	456	275	186
Queue Length 95th (ft)	21	0	#275	188	#645	#797	#492	245
Internal Link Dist (ft)		123			241			150
Turn Bay Length (ft)				250		230	235	
Base Capacity (vph)	77	186	538	741	2282	1017	421	3718
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.06	0.82	0.39	0.94	1.00	0.90	0.49

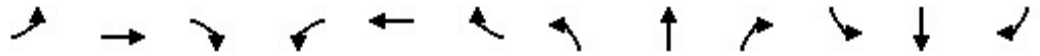
Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 20: Staples Mill Road & Dumbarton Road / Wharfside Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖↗		↖		↑↑↑	↖	↖	↑↑↑	
Traffic Volume (vph)	6	0	10	406	0	263	0	1975	935	350	1668	0
Future Volume (vph)	6	0	10	406	0	263	0	1975	935	350	1668	0
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0		5.0		6.0	6.0	5.0	6.0	
Lane Util. Factor	1.00	1.00		0.97		1.00		0.91	1.00	1.00	0.91	
Frt	1.00	0.85		1.00		0.85		1.00	0.85	1.00	1.00	
Flt Protected	0.95	1.00		0.95		1.00		1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1805	1615		3502		1615		5187	1615	1805	5187	
Flt Permitted	0.95	1.00		0.95		1.00		1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1805	1615		3502		1615		5187	1615	1805	5187	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	7	0	11	441	0	286	0	2147	1016	380	1813	0
RTOR Reduction (vph)	0	11	0	0	0	49	0	0	321	0	0	0
Lane Group Flow (vph)	7	0	0	441	0	237	0	2147	695	380	1813	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Split	NA		Prot		pm+ov		NA	Perm	Prot	NA	
Protected Phases	3	3		4		1		2		1	6	
Permitted Phases						4			2			
Actuated Green, G (s)	2.0	2.0		17.8		44.9		48.1	48.1	27.1	80.2	
Effective Green, g (s)	2.0	2.0		17.8		44.9		48.1	48.1	27.1	80.2	
Actuated g/C Ratio	0.02	0.02		0.15		0.39		0.41	0.41	0.23	0.69	
Clearance Time (s)	5.0	5.0		5.0		5.0		6.0	6.0	5.0	6.0	
Vehicle Extension (s)	2.5	2.5		2.5		2.5		6.0	6.0	2.5	6.0	
Lane Grp Cap (vph)	31	27		537		694		2150	669	421	3586	
v/s Ratio Prot	c0.00	0.00		c0.13		0.08		0.41		c0.21	0.35	
v/s Ratio Perm						0.07			c0.43			
v/c Ratio	0.23	0.01		0.82		0.34		1.00	1.04	0.90	0.51	
Uniform Delay, d1	56.2	56.0		47.6		25.1		33.9	34.0	43.2	8.5	
Progression Factor	1.00	1.00		1.00		1.00		1.00	1.00	1.00	1.00	
Incremental Delay, d2	2.7	0.1		9.6		0.2		19.1	45.4	22.1	0.5	
Delay (s)	58.9	56.1		57.1		25.3		53.0	79.3	65.3	9.0	
Level of Service	E	E		E		C		D	E	E	A	
Approach Delay (s)		57.2			44.6			61.5			18.8	
Approach LOS		E			D			E			B	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			44.1		HCM 2000 Level of Service				D			
HCM 2000 Volume to Capacity ratio			0.94									
Actuated Cycle Length (s)			116.0		Sum of lost time (s)				21.0			
Intersection Capacity Utilization			94.8%		ICU Level of Service				F			
Analysis Period (min)			15									
c Critical Lane Group												

Queues

21: Staples Mill Road & Dickens Road / Ent. to Comcast

10/12/2021



Lane Group	EBL	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	331	331	368	34	30	12	2381	14	2267
v/c Ratio	0.87	0.87	0.81	0.43	0.16	0.16	0.79	0.18	0.74
Control Delay	67.2	67.2	43.5	70.9	1.9	58.1	23.3	59.1	20.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	67.2	67.2	43.5	70.9	1.9	58.1	23.3	59.1	20.1
Queue Length 50th (ft)	248	248	177	25	0	9	470	10	430
Queue Length 95th (ft)	#407	#407	#322	59	0	30	661	32	608
Internal Link Dist (ft)		91		125			466		240
Turn Bay Length (ft)			105		50	450		180	
Base Capacity (vph)	399	399	469	79	186	77	3009	77	3066
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.83	0.78	0.43	0.16	0.16	0.79	0.18	0.74

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 21: Staples Mill Road & Dickens Road / Ent. to Comcast

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	609	0	339	23	8	28	11	2174	17	13	1892	193
Future Volume (vph)	609	0	339	23	8	28	11	2174	17	13	1892	193
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0	
Lane Util. Factor	0.95	0.95	1.00		1.00	1.00	1.00	0.91		1.00	0.91	
Frt	1.00	1.00	0.85		1.00	0.85	1.00	1.00		1.00	0.99	
Flt Protected	0.95	0.95	1.00		0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1715	1715	1615		1833	1615	1805	5181		1805	5115	
Flt Permitted	0.95	0.95	1.00		0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1715	1715	1615		1833	1615	1805	5181		1805	5115	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	662	0	368	25	9	30	12	2363	18	14	2057	210
RTOR Reduction (vph)	0	0	95	0	0	29	0	0	0	0	9	0
Lane Group Flow (vph)	331	331	273	0	34	1	12	2381	0	14	2258	0
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases			4			3						
Actuated Green, G (s)	25.6	25.6	25.6		4.0	4.0	1.0	63.4		2.0	64.4	
Effective Green, g (s)	25.6	25.6	25.6		4.0	4.0	1.0	63.4		2.0	64.4	
Actuated g/C Ratio	0.22	0.22	0.22		0.03	0.03	0.01	0.55		0.02	0.56	
Clearance Time (s)	5.0	5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0	
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5	2.5	6.0		2.5	6.0	
Lane Grp Cap (vph)	378	378	356		63	55	15	2831		31	2839	
v/s Ratio Prot	c0.19	0.19			c0.02		0.01	c0.46		c0.01	0.44	
v/s Ratio Perm			0.17			0.00						
v/c Ratio	0.88	0.88	0.77		0.54	0.02	0.80	0.84		0.45	0.80	
Uniform Delay, d1	43.7	43.7	42.4		55.1	54.1	57.4	22.1		56.5	20.5	
Progression Factor	1.00	1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	19.5	19.5	9.1		6.7	0.1	129.7	3.2		7.4	2.4	
Delay (s)	63.2	63.2	51.5		61.8	54.2	187.1	25.3		63.9	22.9	
Level of Service	E	E	D		E	D	F	C		E	C	
Approach Delay (s)		59.0			58.3			26.1			23.2	
Approach LOS		E			E			C			C	

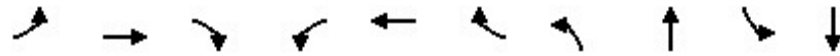
Intersection Summary

HCM 2000 Control Delay	31.2	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.83		
Actuated Cycle Length (s)	116.0	Sum of lost time (s)	21.0
Intersection Capacity Utilization	79.3%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

Queues

22: Brook Road & E Parham Road

10/12/2021






































Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	265	943	142	141	728	358	272	1039	448	985
v/c Ratio	0.68	0.61	0.18	0.52	0.78	0.42	0.70	0.88	0.87	0.70
Control Delay	58.9	36.9	5.9	58.9	46.4	12.3	60.5	52.3	67.7	40.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.9	36.9	5.9	58.9	46.4	12.3	60.5	52.3	67.7	40.0
Queue Length 50th (ft)	101	217	16	54	262	86	104	279	173	238
Queue Length 95th (ft)	141	266	49	85	341	170	147	#374	#284	#310
Internal Link Dist (ft)		437			433			569		731
Turn Bay Length (ft)	415		350	300			400		550	
Base Capacity (vph)	596	1625	831	360	952	844	455	1179	515	1407
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.44	0.58	0.17	0.39	0.76	0.42	0.60	0.88	0.87	0.70

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 22: Brook Road & E Parham Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	  		 	 		 	  		 	  	
Traffic Volume (veh/h)	246	877	132	131	677	333	253	797	169	417	675	241
Future Volume (veh/h)	246	877	132	131	677	333	253	797	169	417	675	241
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1856	1870	1885	1885	1870	1870	1885	1885	1885
Adj Flow Rate, veh/h	265	943	142	141	728	358	272	857	182	448	726	259
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	3	2	1	1	2	2	1	1	1
Cap, veh/h	329	1386	582	201	836	585	333	1285	271	458	1278	450
Arrive On Green	0.10	0.27	0.27	0.06	0.24	0.24	0.10	0.30	0.30	0.13	0.34	0.34
Sat Flow, veh/h	3456	5106	1585	3428	3554	1598	3483	4221	891	3483	3753	1322
Grp Volume(v), veh/h	265	943	142	141	728	358	272	690	349	448	663	322
Grp Sat Flow(s),veh/h/ln	1728	1702	1585	1714	1777	1598	1742	1702	1708	1742	1716	1644
Q Serve(g_s), s	8.9	19.5	7.4	4.8	23.3	21.6	9.0	20.9	21.1	15.1	18.6	19.0
Cycle Q Clear(g_c), s	8.9	19.5	7.4	4.8	23.3	21.6	9.0	20.9	21.1	15.1	18.6	19.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.52	1.00		0.80
Lane Grp Cap(c), veh/h	329	1386	582	201	836	585	333	1037	520	458	1168	560
V/C Ratio(X)	0.81	0.68	0.24	0.70	0.87	0.61	0.82	0.67	0.67	0.98	0.57	0.58
Avail Cap(c_a), veh/h	600	1601	648	363	864	598	458	1037	520	458	1168	560
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	52.3	38.4	26.0	54.5	43.4	30.5	52.4	35.8	35.9	51.1	31.8	31.9
Incr Delay (d2), s/veh	1.8	1.3	0.4	1.7	10.0	2.4	5.8	3.4	6.7	36.4	2.0	4.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.9	8.3	2.8	2.1	11.3	8.6	4.2	9.1	9.7	8.9	8.0	8.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	54.1	39.7	26.3	56.2	53.4	32.9	58.1	39.2	42.6	87.5	33.8	36.2
LnGrp LOS	D	D	C	E	D	C	E	D	D	F	C	D
Approach Vol, veh/h		1350			1227			1311			1433	
Approach Delay, s/veh		41.1			47.7			44.0			51.1	
Approach LOS		D			D			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	22.0	43.2	13.4	39.3	17.8	47.5	17.7	35.0				
Change Period (Y+Rc), s	6.5	* 7.3	6.5	* 7.3	6.5	* 7.3	6.5	7.3				
Max Green Setting (Gmax), s	15.5	* 26	12.5	* 37	15.5	* 26	20.5	28.7				
Max Q Clear Time (g_c+I1), s	17.1	23.1	6.8	21.5	11.0	21.0	10.9	25.3				
Green Ext Time (p_c), s	0.0	2.3	0.1	9.0	0.2	3.3	0.4	2.5				

Intersection Summary

HCM 6th Ctrl Delay	46.1
HCM 6th LOS	D

Notes

\* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Queues

23: Brook Road & Hilliard Road / Hilliard Avenue

10/12/2021



Lane Group	EBL	EBR	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	369	285	81	5	33	227	1128	1005	76
v/c Ratio	0.74	0.49	0.49	no cap	0.13	0.81	0.50	0.79	0.07
Control Delay	57.5	13.4	60.8		1.1	70.6	13.2	36.6	2.8
Queue Delay	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0
Total Delay	57.5	13.4	60.8	Error	1.1	70.6	13.2	36.6	2.8
Queue Length 50th (ft)	138	61	59	0	0	165	243	376	1
Queue Length 95th (ft)	190	114	110	0	0	#286	316	#521	21
Internal Link Dist (ft)				482			335	653	
Turn Bay Length (ft)		275			50	210			115
Base Capacity (vph)	543	607	206	1	280	301	2275	1267	1069
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.47	0.39	5.00	0.12	0.75	0.50	0.79	0.07

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 23: Brook Road & Hilliard Road / Hilliard Avenue

10/13/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗		↖	↖		↖	↖	↖↗			↖↗	↖
Traffic Volume (vph)	351	0	271	82	0	31	216	1010	62	48	906	72
Future Volume (vph)	351	0	271	82	0	31	216	1010	62	48	906	72
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.5		5.5	5.5		5.5	5.5	6.0			6.0	5.5
Lane Util. Factor	0.97		1.00	1.00		1.00	1.00	0.95			0.95	1.00
Frt	1.00		0.85	1.00		0.85	1.00	0.99			1.00	0.85
Flt Protected	0.95		1.00	0.95		1.00	0.95	1.00			1.00	1.00
Satd. Flow (prot)	3467		1599	1805		1455	1787	3545			3601	1615
Flt Permitted	0.95		1.00	0.95		1.00	0.95	1.00			0.80	1.00
Satd. Flow (perm)	3467		1599	1805		1455	1787	3545			2891	1615
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	369	0	285	86	0	33	227	1063	65	51	954	76
RTOR Reduction (vph)	0	0	86	0	0	30	0	3	0	0	0	32
Lane Group Flow (vph)	369	0	199	86	0	3	227	1125	0	0	1005	44
Heavy Vehicles (%)	1%	0%	1%	0%	0%	11%	1%	1%	0%	0%	0%	0%
Turn Type	Prot		pt+ov	Prot		Prot	Prot	NA		Perm	NA	pt+ov
Protected Phases	3		3 5	4		4	5	2			6	3 6
Permitted Phases										6		
Actuated Green, G (s)	17.1		35.5	9.5		9.5	18.4	74.4			50.5	73.6
Effective Green, g (s)	17.1		35.5	9.5		9.5	18.4	74.4			50.5	67.6
Actuated g/C Ratio	0.14		0.30	0.08		0.08	0.16	0.63			0.43	0.57
Clearance Time (s)	5.5			5.5		5.5	5.5	6.0			6.0	
Vehicle Extension (s)	3.5			3.5		3.5	3.0	7.0			7.0	
Lane Grp Cap (vph)	502		481	145		117	278	2235			1237	925
v/s Ratio Prot	c0.11		0.12	c0.05		0.00	c0.13	0.32				0.03
v/s Ratio Perm											c0.35	
v/c Ratio	0.74		0.41	0.59		0.02	0.82	0.50			0.81	0.05
Uniform Delay, d1	48.3		32.9	52.4		50.0	48.2	11.8			29.6	11.1
Progression Factor	1.00		1.00	1.00		1.00	1.00	1.00			1.00	1.00
Incremental Delay, d2	5.7		0.7	6.7		0.1	16.7	0.8			5.9	0.0
Delay (s)	54.0		33.6	59.1		50.1	64.9	12.6			35.5	11.1
Level of Service	D		C	E		D	E	B			D	B
Approach Delay (s)		45.1			56.6			21.4			33.8	
Approach LOS		D			E			C			C	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			31.7									HCM 2000 Level of Service C
HCM 2000 Volume to Capacity ratio			0.78									
Actuated Cycle Length (s)			118.0								22.5	
Intersection Capacity Utilization			80.9%									ICU Level of Service D
Analysis Period (min)			15									
c Critical Lane Group												

Queues

24: Brook Road & Dumbarton Road / Azalea Avenue

10/12/2021



Lane Group	EBL	EBT	WBL	WBT	WBR	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	153	893	107	447	435	610	214	534	468	112
v/c Ratio	0.89	0.96	0.72	0.51	0.61	0.83	0.45	1.19	0.51	0.23
Control Delay	103.5	68.3	84.8	44.6	7.7	59.9	11.2	148.8	44.0	6.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	103.5	68.3	84.8	44.6	7.7	59.9	11.2	148.8	44.0	6.0
Queue Length 50th (ft)	129	391	89	172	0	261	14	~542	178	0
Queue Length 95th (ft)	#256	#527	#179	227	91	#335	85	#765	234	39
Internal Link Dist (ft)		442		279		310			329	
Turn Bay Length (ft)	210		80					150		150
Base Capacity (vph)	176	931	149	879	710	738	477	448	915	496
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.87	0.96	0.72	0.51	0.61	0.83	0.45	1.19	0.51	0.23

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

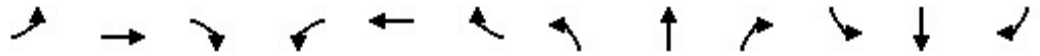
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 24: Brook Road & Dumbarton Road / Azalea Avenue

10/12/2021



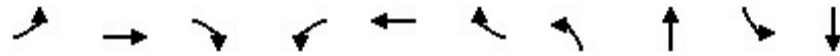
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	150	822	53	105	438	426	67	531	210	523	459	110
Future Volume (vph)	150	822	53	105	438	426	67	531	210	523	459	110
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	5.0		5.8	5.0	5.0		7.1	7.1	7.0	7.0	7.0
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00
Frbp, ped/bikes	1.00	1.00		1.00	1.00	0.98		1.00	0.99	1.00	1.00	0.98
Flpb, ped/bikes	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.99		1.00	1.00	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.99	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1805	3541		1736	3539	1544		3558	1560	1770	3610	1589
Flt Permitted	0.95	1.00		0.95	1.00	1.00		0.99	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1805	3541		1736	3539	1544		3558	1560	1770	3610	1589
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	153	839	54	107	447	435	68	542	214	534	468	112
RTOR Reduction (vph)	0	4	0	0	0	327	0	0	154	0	0	84
Lane Group Flow (vph)	153	889	0	107	447	108	0	610	60	534	468	28
Confl. Peds. (#/hr)	2		1	1		2	3		2	2		3
Heavy Vehicles (%)	0%	1%	0%	4%	2%	3%	0%	1%	2%	2%	0%	0%
Turn Type	Prot	NA		Prot	NA	Perm	Split	NA	Perm	Split	NA	Perm
Protected Phases	3	8		7	4		1	1		2	2	
Permitted Phases						4			1			2
Actuated Green, G (s)	12.4	34.1		11.1	32.3	32.3		27.0	27.0	33.0	33.0	33.0
Effective Green, g (s)	12.4	34.1		11.1	32.3	32.3		27.0	27.0	33.0	33.0	33.0
Actuated g/C Ratio	0.10	0.26		0.09	0.25	0.25		0.21	0.21	0.25	0.25	0.25
Clearance Time (s)	6.3	5.0		5.8	5.0	5.0		7.1	7.1	7.0	7.0	7.0
Vehicle Extension (s)	0.2	0.2		0.2	0.2	0.2		0.2	0.2	0.2	0.2	0.2
Lane Grp Cap (vph)	172	928		148	878	383		738	323	448	915	403
v/s Ratio Prot	c0.08	c0.25		0.06	0.13			c0.17		c0.30	0.13	
v/s Ratio Perm						0.07			0.04			0.02
v/c Ratio	0.89	0.96		0.72	0.51	0.28		0.83	0.19	1.19	0.51	0.07
Uniform Delay, d1	58.2	47.3		58.0	42.1	39.5		49.3	42.5	48.5	41.6	36.9
Progression Factor	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	37.6	21.0		13.7	2.1	1.8		10.3	1.3	106.6	2.0	0.3
Delay (s)	95.8	68.3		71.7	44.2	41.4		59.6	43.8	155.2	43.7	37.2
Level of Service	F	E		E	D	D		E	D	F	D	D
Approach Delay (s)		72.3			45.9			55.5			96.5	
Approach LOS		E			D			E			F	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			69.0				HCM 2000 Level of Service			E		
HCM 2000 Volume to Capacity ratio			1.01									
Actuated Cycle Length (s)			130.1				Sum of lost time (s)		25.4			
Intersection Capacity Utilization			105.8%				ICU Level of Service		G			
Analysis Period (min)			15									

c Critical Lane Group

Queues

25: Springfield Road & Gaskins Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	306	995	138	350	843	215	98	1229	265	871
v/c Ratio	0.98	1.50	0.31	1.01	1.21	0.36	0.66	1.19	0.91	0.71
Control Delay	92.1	265.0	4.7	96.2	148.1	16.6	70.9	130.0	82.6	35.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	92.1	265.0	4.7	96.2	148.1	16.6	70.9	130.0	82.6	35.2
Queue Length 50th (ft)	247	~556	0	~261	~397	65	70	~534	192	283
Queue Length 95th (ft)	#443	#692	31	#454	#524	125	127	#672	#342	361
Internal Link Dist (ft)		455			617			1015		812
Turn Bay Length (ft)	225		135	285		165	210		450	
Base Capacity (vph)	313	665	450	348	696	614	174	1029	300	1224
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.98	1.50	0.31	1.01	1.21	0.35	0.56	1.19	0.88	0.71

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.



























Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 25: Springfield Road & Gaskins Road

10/12/2021

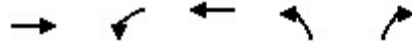
													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		 			 			 			 		
Traffic Volume (vph)	313	884	127	322	776	198	90	637	494	244	624	178	
Future Volume (vph)	313	884	127	322	776	198	90	637	494	244	624	178	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0		5.0	6.0		
Lane Util. Factor	0.91	0.91	1.00	1.00	0.95	1.00	1.00	0.95		1.00	0.95		
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00		1.00	1.00		
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00		
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.93		1.00	0.97		
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (prot)	1626	3451	1615	1805	3610	1572	1805	3373		1805	3465		
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (perm)	1626	3451	1615	1805	3610	1572	1805	3373		1805	3465		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	340	961	138	350	843	215	98	692	537	265	678	193	
RTOR Reduction (vph)	0	0	111	0	0	50	0	122	0	0	22	0	
Lane Group Flow (vph)	306	995	27	350	843	165	98	1107	0	265	849	0	
Confl. Peds. (#/hr)	1					1	1					1	
Heavy Vehicles (%)	1%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	2%	
Turn Type	Split	NA	Perm	Split	NA	pm+ov	Prot	NA		Prot	NA		
Protected Phases	3	3		4	4	1	5	2		1	6		
Permitted Phases			3			4							
Actuated Green, G (s)	22.0	22.0	22.0	22.0	22.0	40.3	9.5	30.7		18.3	39.5		
Effective Green, g (s)	22.0	22.0	22.0	22.0	22.0	40.3	9.5	30.7		18.3	39.5		
Actuated g/C Ratio	0.19	0.19	0.19	0.19	0.19	0.35	0.08	0.27		0.16	0.35		
Clearance Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	6.0		5.0	6.0		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	2.0	2.0	6.0		2.0	6.0		
Lane Grp Cap (vph)	313	665	311	348	696	624	150	908		289	1200		
v/s Ratio Prot	0.19	c0.29		0.19	c0.23	0.04	0.05	c0.33		c0.15	0.24		
v/s Ratio Perm			0.02			0.06							
v/c Ratio	0.98	1.50	0.09	1.01	1.21	0.26	0.65	1.22		0.92	0.71		
Uniform Delay, d1	45.8	46.0	37.7	46.0	46.0	26.3	50.7	41.6		47.1	32.2		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00		
Incremental Delay, d2	44.3	231.2	0.1	49.7	108.1	0.1	7.5	108.6		31.3	3.5		
Delay (s)	90.1	277.2	37.9	95.7	154.1	26.4	58.2	150.3		78.4	35.8		
Level of Service	F	F	D	F	F	C	E	F		E	D		
Approach Delay (s)		214.4			120.1			143.5			45.7		
Approach LOS		F			F			F			D		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			135.6	HCM 2000 Level of Service						F			
HCM 2000 Volume to Capacity ratio			1.22										
Actuated Cycle Length (s)			114.0	Sum of lost time (s)						21.0			
Intersection Capacity Utilization			110.4%	ICU Level of Service						H			
Analysis Period (min)			15										

c Critical Lane Group

Queues

26: West End Drive & Hungary Road

10/12/2021



Lane Group	EBT	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1599	327	1003	463	435
v/c Ratio	1.01	1.00	0.43	0.96	0.65
Control Delay	58.1	87.2	11.5	78.0	15.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	58.1	87.2	11.5	78.0	15.9
Queue Length 50th (ft)	~684	218	195	376	76
Queue Length 95th (ft)	#859	#416	237	#589	196
Internal Link Dist (ft)	202		219	505	
Turn Bay Length (ft)				90	
Base Capacity (vph)	1589	328	2345	483	667
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	1.01	1.00	0.43	0.96	0.65

Intersection Summary

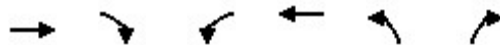
~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 26: West End Drive & Hungary Road

10/12/2021



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑		↵	↑↑	↵	↵
Traffic Volume (vph)	1191	280	301	923	426	400
Future Volume (vph)	1191	280	301	923	426	400
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.5		5.5	5.5	5.0	5.0
Lane Util. Factor	0.95		1.00	0.95	1.00	1.00
Frpb, ped/bikes	1.00		1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00		1.00	1.00	1.00	1.00
Frt	0.97		1.00	1.00	1.00	0.85
Flt Protected	1.00		0.95	1.00	0.95	1.00
Satd. Flow (prot)	3451		1805	3610	1805	1615
Flt Permitted	1.00		0.06	1.00	0.95	1.00
Satd. Flow (perm)	3451		119	3610	1805	1615
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1295	304	327	1003	463	435
RTOR Reduction (vph)	12	0	0	0	0	235
Lane Group Flow (vph)	1587	0	327	1003	463	200
Confl. Peds. (#/hr)					1	
Heavy Vehicles (%)	2%	0%	0%	0%	0%	0%
Turn Type	NA		pm+pt	NA	Prot	Perm
Protected Phases	2		1	6	4	
Permitted Phases			6			4
Actuated Green, G (s)	58.1		82.5	82.5	34.0	34.0
Effective Green, g (s)	58.1		82.5	82.5	34.0	34.0
Actuated g/C Ratio	0.46		0.65	0.65	0.27	0.27
Clearance Time (s)	5.5		5.5	5.5	5.0	5.0
Vehicle Extension (s)	6.0		3.0	6.0	3.0	3.0
Lane Grp Cap (vph)	1578		328	2345	483	432
v/s Ratio Prot	0.46		c0.15	0.28	c0.26	
v/s Ratio Perm			c0.50			0.12
v/c Ratio	1.01		1.00	0.43	0.96	0.46
Uniform Delay, d1	34.5		43.8	10.8	45.8	38.9
Progression Factor	1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2	24.1		48.5	0.4	30.3	0.8
Delay (s)	58.5		92.3	11.2	76.1	39.7
Level of Service	E		F	B	E	D
Approach Delay (s)	58.5			31.1	58.4	
Approach LOS	E			C	E	

### Intersection Summary

HCM 2000 Control Delay	49.0	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	1.04		
Actuated Cycle Length (s)	127.0	Sum of lost time (s)	19.0
Intersection Capacity Utilization	95.5%	ICU Level of Service	F
Analysis Period (min)	15		

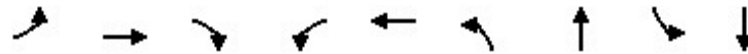
c Critical Lane Group



Queues

27: Woodman Road & Hungary Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	434	380	243	185	499	224	1000	77	915
v/c Ratio	1.09	0.61	0.35	0.53	1.14	0.96	0.94	0.34	0.85
Control Delay	106.0	39.0	5.2	27.6	130.2	100.7	57.1	52.4	45.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	106.0	39.0	5.2	27.6	130.2	100.7	57.1	52.4	45.4
Queue Length 50th (ft)	~329	249	0	86	~452	175	388	55	328
Queue Length 95th (ft)	#535	356	57	136	#667	#332	#526	104	413
Internal Link Dist (ft)		244			266		277		252
Turn Bay Length (ft)	240			75		260		200	
Base Capacity (vph)	397	622	692	362	436	234	1061	265	1087
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.09	0.61	0.35	0.51	1.14	0.96	0.94	0.29	0.84

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

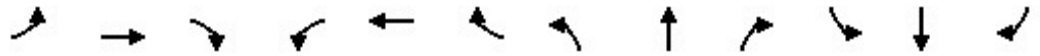
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 27: Woodman Road & Hungary Road

10/12/2021

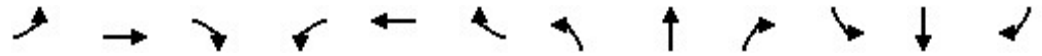


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	404	353	226	172	402	62	208	784	146	72	589	262
Future Volume (veh/h)	404	353	226	172	402	62	208	784	146	72	589	262
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1885	1885	1900	1900	1900	1885	1870	1870	1900	1856	1856
Adj Flow Rate, veh/h	434	380	243	185	432	67	224	843	157	77	633	282
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	1	1	0	0	0	1	2	2	0	3	3
Cap, veh/h	398	618	523	358	377	58	239	884	165	230	707	315
Arrive On Green	0.19	0.33	0.33	0.09	0.23	0.23	0.13	0.30	0.30	0.13	0.30	0.30
Sat Flow, veh/h	1810	1885	1598	1810	1606	249	1795	2990	557	1810	2372	1056
Grp Volume(v), veh/h	434	380	243	185	0	499	224	501	499	77	470	445
Grp Sat Flow(s),veh/h/ln	1810	1885	1598	1810	0	1855	1795	1777	1770	1810	1763	1665
Q Serve(g_s), s	22.0	20.0	14.2	9.0	0.0	27.7	14.6	32.7	32.7	4.6	30.2	30.2
Cycle Q Clear(g_c), s	22.0	20.0	14.2	9.0	0.0	27.7	14.6	32.7	32.7	4.6	30.2	30.2
Prop In Lane	1.00		1.00	1.00		0.13	1.00		0.31	1.00		0.63
Lane Grp Cap(c), veh/h	398	618	523	358	0	435	239	525	523	230	526	497
V/C Ratio(X)	1.09	0.62	0.46	0.52	0.00	1.15	0.94	0.95	0.95	0.34	0.90	0.90
Avail Cap(c_a), veh/h	398	618	523	358	0	435	239	525	523	270	554	523
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	36.2	33.4	31.5	30.2	0.0	45.2	50.7	40.8	40.8	47.0	39.7	39.7
Incr Delay (d2), s/veh	71.7	1.8	0.6	0.6	0.0	89.9	41.1	28.1	28.2	1.2	17.0	17.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	15.8	9.4	5.6	4.0	0.0	23.5	9.3	18.2	18.2	2.1	15.4	14.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	107.9	35.3	32.1	30.8	0.0	135.1	91.8	68.9	69.0	48.2	56.7	57.5
LnGrp LOS	F	D	C	C	A	F	F	E	E	D	E	E
Approach Vol, veh/h		1057			684			1224			992	
Approach Delay, s/veh		64.4			106.9			73.2			56.4	
Approach LOS		E			F			E			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	20.0	39.9	15.0	43.2	19.7	40.2	26.0	32.2				
Change Period (Y+Rc), s	5.0	5.0	4.0	4.5	4.0	5.0	4.0	4.5				
Max Green Setting (Gmax), s	17.6	34.2	11.0	38.7	15.7	37.1	22.0	27.7				
Max Q Clear Time (g_c+I1), s	6.6	34.7	11.0	22.0	16.6	32.2	24.0	29.7				
Green Ext Time (p_c), s	0.2	0.0	0.0	2.9	0.0	3.0	0.0	0.0				
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay				72.4								
HCM 6th LOS				E								

Queues

28: Hungary Spring Road & E Parham Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	279	932	46	202	815	129	87	533	254	70	292	208
v/c Ratio	0.82	0.75	0.07	0.68	0.68	0.20	0.22	0.60	0.44	0.24	0.33	0.38
Control Delay	35.9	26.4	0.2	26.0	24.9	4.6	16.3	27.3	5.9	16.8	23.7	5.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.9	26.4	0.2	26.0	24.9	4.6	16.3	27.3	5.9	16.8	23.7	5.8
Queue Length 50th (ft)	72	200	0	49	171	0	26	116	0	20	59	0
Queue Length 95th (ft)	#213	292	0	#143	252	33	53	164	52	45	92	47
Internal Link Dist (ft)		568			685			340			433	
Turn Bay Length (ft)	225		200	400		305	230		190	330		275
Base Capacity (vph)	341	1358	690	299	1317	681	395	1200	695	290	1188	670
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.69	0.07	0.68	0.62	0.19	0.22	0.44	0.37	0.24	0.25	0.31

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 28: Hungary Spring Road & E Parham Road

10/12/2021

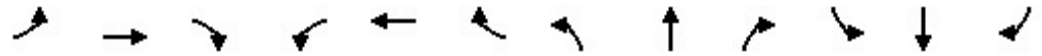


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↶	↷	↷	↶	↷	↷	↶	↷	↷	↶	↷	↷
Traffic Volume (veh/h)	271	904	45	196	791	125	84	517	246	68	283	202
Future Volume (veh/h)	271	904	45	196	791	125	84	517	246	68	283	202
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1885	1900	1900	1885	1900	1870	1900	1870	1900	1885	1885
Adj Flow Rate, veh/h	279	932	46	202	815	129	87	533	254	70	292	208
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	0	1	0	0	1	0	2	0	2	0	1	1
Cap, veh/h	410	1254	563	363	1174	527	342	833	366	257	788	352
Arrive On Green	0.12	0.35	0.35	0.10	0.33	0.33	0.05	0.23	0.23	0.04	0.22	0.22
Sat Flow, veh/h	1810	3582	1609	1810	3582	1609	1781	3610	1585	1810	3582	1598
Grp Volume(v), veh/h	279	932	46	202	815	129	87	533	254	70	292	208
Grp Sat Flow(s),veh/h/ln	1810	1791	1609	1810	1791	1609	1781	1805	1585	1810	1791	1598
Q Serve(g_s), s	6.5	14.9	1.3	4.7	12.9	3.8	2.4	8.7	9.6	1.9	4.5	7.6
Cycle Q Clear(g_c), s	6.5	14.9	1.3	4.7	12.9	3.8	2.4	8.7	9.6	1.9	4.5	7.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	410	1254	563	363	1174	527	342	833	366	257	788	352
V/C Ratio(X)	0.68	0.74	0.08	0.56	0.69	0.24	0.25	0.64	0.69	0.27	0.37	0.59
Avail Cap(c_a), veh/h	410	1442	648	381	1398	628	396	1271	558	331	1261	562
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	14.0	18.7	14.2	14.2	19.1	16.0	18.3	22.7	23.0	19.0	21.6	22.8
Incr Delay (d2), s/veh	4.5	2.3	0.1	1.6	1.6	0.4	0.4	0.8	2.4	0.6	0.3	1.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.8	6.0	0.4	1.9	5.2	1.4	1.0	3.6	3.6	0.8	1.8	2.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	18.5	20.9	14.3	15.8	20.7	16.5	18.7	23.5	25.4	19.5	21.9	24.4
LnGrp LOS	B	C	B	B	C	B	B	C	C	B	C	C
Approach Vol, veh/h		1257			1146			874			570	
Approach Delay, s/veh		20.1			19.4			23.6			22.5	
Approach LOS		C			B			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.6	27.9	6.8	20.1	12.0	26.4	7.5	19.4				
Change Period (Y+Rc), s	4.0	5.0	4.0	5.0	4.0	5.0	4.0	5.0				
Max Green Setting (Gmax), s	7.2	26.3	5.5	23.0	8.0	25.5	5.5	23.0				
Max Q Clear Time (g_c+I1), s	6.7	16.9	3.9	11.6	8.5	14.9	4.4	9.6				
Green Ext Time (p_c), s	0.0	5.9	0.0	3.5	0.0	6.1	0.0	2.2				
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay				21.1								
HCM 6th LOS				C								

Queues

29: Woodman Road & E Parham Road

10/12/2021



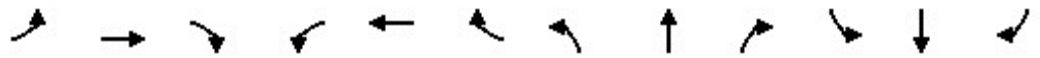
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	563	1158	79	38	838	185	60	461	38	217	331	465
v/c Ratio	0.99	0.64	0.09	0.16	0.92	0.34	0.20	0.71	0.09	0.92	0.42	0.51
Control Delay	61.0	19.0	1.5	12.7	48.9	6.2	23.2	40.2	0.4	68.9	31.9	11.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.0	19.0	1.5	12.7	48.9	6.2	23.2	40.2	0.4	68.9	31.9	11.1
Queue Length 50th (ft)	~274	269	0	9	246	0	24	127	0	94	85	106
Queue Length 95th (ft)	#491	345	12	21	#363	50	52	179	0	#191	126	188
Internal Link Dist (ft)		299			266			659				468
Turn Bay Length (ft)	430		75	230		245	264		55	346		95
Base Capacity (vph)	567	1822	833	242	912	550	299	730	474	237	820	910
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.99	0.64	0.09	0.16	0.92	0.34	0.20	0.63	0.08	0.92	0.40	0.51

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 29: Woodman Road & E Parham Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑	↗
Traffic Volume (veh/h)	535	1100	75	36	796	176	57	438	36	206	314	442
Future Volume (veh/h)	535	1100	75	36	796	176	57	438	36	206	314	442
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1885	1811	1900	1885	1900	1870	1885	1900	1870	1885	1856
Adj Flow Rate, veh/h	563	1158	0	38	838	0	60	461	0	217	331	465
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	1	6	0	1	0	2	1	0	2	1	3
Cap, veh/h	587	1809		249	912		248	648		280	796	780
Arrive On Green	0.27	0.51	0.00	0.02	0.25	0.00	0.04	0.18	0.00	0.08	0.22	0.22
Sat Flow, veh/h	1781	3582	1535	1810	3582	1610	1781	3582	1610	1781	3582	1572
Grp Volume(v), veh/h	563	1158	0	38	838	0	60	461	0	217	331	465
Grp Sat Flow(s),veh/h/ln	1781	1791	1535	1810	1791	1610	1781	1791	1610	1781	1791	1572
Q Serve(g_s), s	22.3	20.7	0.0	1.4	20.0	0.0	2.4	10.6	0.0	7.0	6.9	18.6
Cycle Q Clear(g_c), s	22.3	20.7	0.0	1.4	20.0	0.0	2.4	10.6	0.0	7.0	6.9	18.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	587	1809		249	912		248	648		280	796	780
V/C Ratio(X)	0.96	0.64		0.15	0.92		0.24	0.71		0.77	0.42	0.60
Avail Cap(c_a), veh/h	587	1809		321	919		291	735		280	796	780
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	22.2	15.9	0.0	23.4	31.8	0.0	27.7	33.8	0.0	29.4	29.2	15.8
Incr Delay (d2), s/veh	27.1	0.9	0.0	0.3	14.2	0.0	0.5	2.8	0.0	12.7	0.3	1.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	13.0	8.1	0.0	0.6	10.1	0.0	1.0	4.8	0.0	2.1	3.0	6.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	49.3	16.8	0.0	23.6	46.0	0.0	28.2	36.5	0.0	42.1	29.6	17.1
LnGrp LOS	D	B		C	D		C	D		D	C	B
Approach Vol, veh/h		1721	A		876	A		521	A		1013	
Approach Delay, s/veh		27.4			45.0			35.6			26.5	
Approach LOS		C			D			D			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	28.0	28.3	11.0	20.4	6.0	50.3	7.4	24.0				
Change Period (Y+Rc), s	4.0	6.0	4.0	4.5	4.0	6.0	4.0	4.5				
Max Green Setting (Gmax), s	24.0	22.5	7.0	18.0	5.5	41.0	5.5	19.5				
Max Q Clear Time (g_c+I1), s	24.3	22.0	9.0	12.6	3.4	22.7	4.4	20.6				
Green Ext Time (p_c), s	0.0	0.4	0.0	1.4	0.0	10.5	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay	32.0
HCM 6th LOS	C

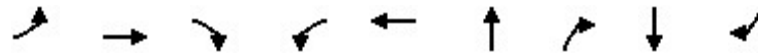
Notes

Unsignalized Delay for [NBR, EBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Queues

30: Bethlehem Road & Glenside Drive

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	NBR	SBT	SBR
Lane Group Flow (vph)	156	1007	91	106	836	227	127	146	74
v/c Ratio	0.67	0.99	0.16	0.48	0.64	0.75	0.30	1.90	0.15
Control Delay	33.7	57.5	2.5	24.1	27.7	45.4	6.7	474.1	1.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.7	57.5	2.5	24.1	27.7	45.4	6.7	474.1	1.4
Queue Length 50th (ft)	50	~295	0	33	130	104	0	~113	0
Queue Length 95th (ft)	#106	#424	15	67	175	#186	37	#226	6
Internal Link Dist (ft)		521			373	430		331	
Turn Bay Length (ft)	275			250			130		610
Base Capacity (vph)	234	1015	556	223	1380	362	477	77	496
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.67	0.99	0.16	0.48	0.61	0.63	0.27	1.90	0.15

Intersection Summary

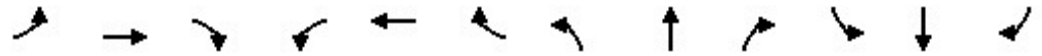
~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 30: Bethlehem Road & Glenside Drive

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	147	947	86	100	730	55	83	131	119	79	58	70
Future Volume (vph)	147	947	86	100	730	55	83	131	119	79	58	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.5	4.5	4.0	4.5			4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91			1.00	1.00		1.00	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00	0.98		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00	1.00		1.00	1.00
Frt	1.00	1.00	0.85	1.00	0.99			1.00	0.85		1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00			0.98	1.00		0.97	1.00
Satd. Flow (prot)	1805	3574	1615	1770	5079			1853	1575		1823	1615
Flt Permitted	0.20	1.00	1.00	0.20	1.00			0.81	1.00		0.17	1.00
Satd. Flow (perm)	381	3574	1615	363	5079			1529	1575		318	1615
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	156	1007	91	106	777	59	88	139	127	84	62	74
RTOR Reduction (vph)	0	0	65	0	10	0	0	0	102	0	0	56
Lane Group Flow (vph)	156	1007	26	106	826	0	0	227	25	0	146	18
Confl. Peds. (#/hr)	3					3			2	2		
Heavy Vehicles (%)	0%	1%	0%	2%	1%	0%	0%	1%	1%	0%	3%	0%
Turn Type	pm+pt	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	1	6		5	2			4			3	
Permitted Phases	6		6	2			4		4	3		3
Actuated Green, G (s)	27.2	21.7	21.7	24.8	20.5			15.1	15.1		18.6	18.6
Effective Green, g (s)	27.2	21.7	21.7	24.8	20.5			15.1	15.1		18.6	18.6
Actuated g/C Ratio	0.35	0.28	0.28	0.32	0.27			0.20	0.20		0.24	0.24
Clearance Time (s)	4.0	4.5	4.5	4.0	4.5			4.5	4.5		4.5	4.5
Vehicle Extension (s)	2.0	4.5	4.5	2.0	4.5			3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	235	1004	453	194	1348			299	308		76	389
v/s Ratio Prot	c0.05	c0.28		0.03	0.16							
v/s Ratio Perm	0.19		0.02	0.14				c0.15	0.02		c0.46	0.01
v/c Ratio	0.66	1.00	0.06	0.55	0.61			0.76	0.08		1.92	0.05
Uniform Delay, d1	18.3	27.8	20.3	20.8	24.9			29.3	25.4		29.3	22.5
Progression Factor	1.00	1.00	1.00	1.00	1.00			1.00	1.00		1.00	1.00
Incremental Delay, d2	5.4	29.1	0.1	1.7	1.1			10.6	0.1		459.1	0.0
Delay (s)	23.7	56.9	20.4	22.5	25.9			39.9	25.5		488.4	22.5
Level of Service	C	E	C	C	C			D	C		F	C
Approach Delay (s)		50.1			25.6			34.7			331.7	
Approach LOS		D			C			C			F	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			62.1			HCM 2000 Level of Service			E			
HCM 2000 Volume to Capacity ratio			1.21									
Actuated Cycle Length (s)			77.2			Sum of lost time (s)			17.5			
Intersection Capacity Utilization			65.2%			ICU Level of Service			C			
Analysis Period (min)			15									

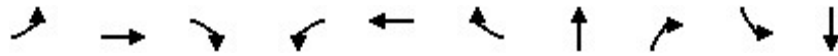
c Critical Lane Group



Queues

31: Hermitage Road & Hilliard Road / Glenside Drive

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	140	1012	56	29	718	434	194	51	411	260
v/c Ratio	0.85	0.82	0.08	0.31	0.81	0.60	0.69	0.13	1.00	0.61
Control Delay	85.6	38.0	0.2	54.5	42.8	7.0	52.3	0.6	85.4	34.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	85.6	38.0	0.2	54.5	42.8	7.0	52.3	0.6	85.4	34.9
Queue Length 50th (ft)	87	323	0	18	221	0	114	0	~261	120
Queue Length 95th (ft)	#202	#477	0	48	#318	79	188	0	#470	210
Internal Link Dist (ft)		315			426		221			272
Turn Bay Length (ft)			35	415		415		45	215	
Base Capacity (vph)	164	1234	671	93	891	728	344	450	409	429
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.85	0.82	0.08	0.31	0.81	0.60	0.56	0.11	1.00	0.61

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
























Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 31: Hermitage Road & Hilliard Road / Glenside Drive

10/12/2021

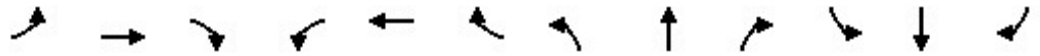
												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	132	951	53	27	675	408	59	123	48	386	123	121
Future Volume (vph)	132	951	53	27	675	408	59	123	48	386	123	121
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.5	5.5	5.5	6.2	5.6	5.6		6.9	6.9	8.1	8.1	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00		1.00	1.00	1.00	1.00	
Frbp, ped/bikes	1.00	1.00	0.98	1.00	1.00	1.00		1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85		1.00	0.85	1.00	0.93	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00		0.98	1.00	0.95	1.00	
Satd. Flow (prot)	1787	3610	1580	1805	3574	1615		1852	1615	1805	1741	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00		0.98	1.00	0.95	1.00	
Satd. Flow (perm)	1787	3610	1580	1805	3574	1615		1852	1615	1805	1741	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	140	1012	56	29	718	434	63	131	51	411	131	129
RTOR Reduction (vph)	0	0	37	0	0	317	0	0	43	0	35	0
Lane Group Flow (vph)	140	1012	19	29	718	117	0	194	8	411	225	0
Confl. Peds. (#/hr)			1	1								
Heavy Vehicles (%)	1%	0%	0%	0%	1%	0%	3%	0%	0%	0%	0%	2%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Split	NA	Perm	Split	NA	
Protected Phases	1	6		5	2		3	3		4	4	
Permitted Phases			6			2			3			
Actuated Green, G (s)	8.9	33.1	33.1	2.9	26.7	26.7		14.6	14.6	21.9	21.9	
Effective Green, g (s)	8.9	33.1	33.1	2.9	26.7	26.7		14.6	14.6	21.9	21.9	
Actuated g/C Ratio	0.09	0.33	0.33	0.03	0.27	0.27		0.15	0.15	0.22	0.22	
Clearance Time (s)	6.5	5.5	5.5	6.2	5.6	5.6		6.9	6.9	8.1	8.1	
Vehicle Extension (s)	3.0	6.0	6.0	3.0	6.0	6.0		3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	160	1204	527	52	961	434		272	237	398	384	
v/s Ratio Prot	c0.08	c0.28		0.02	0.20			c0.10		c0.23	0.13	
v/s Ratio Perm			0.01			0.07			0.00			
v/c Ratio	0.88	0.84	0.04	0.56	0.75	0.27		0.71	0.03	1.03	0.59	
Uniform Delay, d1	44.6	30.6	22.3	47.5	33.2	28.6		40.3	36.2	38.7	34.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	
Incremental Delay, d2	37.4	6.3	0.1	12.3	4.3	0.9		8.6	0.1	53.8	2.3	
Delay (s)	82.0	36.9	22.4	59.8	37.5	29.5		48.9	36.3	92.4	36.9	
Level of Service	F	D	C	E	D	C		D	D	F	D	
Approach Delay (s)		41.5			35.1			46.2			70.9	
Approach LOS		D			D			D			E	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			45.5				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			0.92									
Actuated Cycle Length (s)			99.2				Sum of lost time (s)		27.1			
Intersection Capacity Utilization			83.8%				ICU Level of Service		E			
Analysis Period (min)			15									

c Critical Lane Group

Queues

32: Lakeside Avenue & Hilliard Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	334	587	500	68	258	13	493	220	89	5	223	400
v/c Ratio	0.92	0.55	0.47	0.52	0.42	0.03	0.68	0.56	0.17	0.01	0.64	0.49
Control Delay	83.2	39.5	7.5	75.1	49.3	0.2	53.4	53.6	1.7	46.4	58.7	18.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	83.2	39.5	7.5	75.1	49.3	0.2	53.4	53.6	1.7	46.4	58.7	18.7
Queue Length 50th (ft)	294	227	87	58	105	0	202	170	0	4	179	154
Queue Length 95th (ft)	#549	299	176	116	152	0	287	279	9	16	285	274
Internal Link Dist (ft)		216			777			626			561	
Turn Bay Length (ft)	150		230	115		200	130		90	265		
Base Capacity (vph)	362	1279	1096	166	920	557	834	452	616	419	437	803
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.92	0.46	0.46	0.41	0.28	0.02	0.59	0.49	0.14	0.01	0.51	0.50





























Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 32: Lakeside Avenue & Hilliard Road

10/12/2021

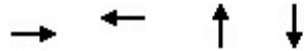
												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 		 				 	
Traffic Volume (vph)	324	569	485	66	250	13	478	213	86	5	216	388
Future Volume (vph)	324	569	485	66	250	13	478	213	86	5	216	388
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	8.2	5.5	5.5	7.3	6.1	6.1	8.3	8.3	8.3	5.5	5.5	5.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00	1.00	1.00	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1752	3374	1583	1719	3574	1594	3467	1881	1615	1805	1881	1599
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1752	3374	1583	1719	3574	1594	3467	1881	1615	1805	1881	1599
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	334	587	500	68	258	13	493	220	89	5	223	400
RTOR Reduction (vph)	0	0	126	0	0	11	0	0	63	0	0	80
Lane Group Flow (vph)	334	587	374	68	258	2	493	220	26	5	223	320
Confl. Peds. (#/hr)	1					1						
Heavy Vehicles (%)	3%	7%	2%	5%	1%	0%	1%	1%	0%	0%	1%	1%
Turn Type	Prot	NA	pt+ov	Prot	NA	Perm	Split	NA	pt+ov	Split	NA	pt+ov
Protected Phases	1	6	6 3	5	2		3	3	3 5	4	4	4 1
Permitted Phases						2						
Actuated Green, G (s)	26.4	40.3	72.4	9.8	22.2	22.2	26.6	26.6	36.4	23.7	23.7	55.6
Effective Green, g (s)	26.4	40.3	72.4	9.8	22.2	22.2	26.6	26.6	36.4	23.7	23.7	55.6
Actuated g/C Ratio	0.21	0.32	0.57	0.08	0.17	0.17	0.21	0.21	0.29	0.19	0.19	0.44
Clearance Time (s)	8.2	5.5		7.3	6.1	6.1	8.3	8.3		5.5	5.5	
Vehicle Extension (s)	3.0	5.0		3.0	5.0	5.0	3.0	3.0		5.0	5.0	
Lane Grp Cap (vph)	364	1070	902	132	624	278	726	393	462	336	351	700
v/s Ratio Prot	c0.19	c0.17	0.24	0.04	0.07		c0.14	0.12	0.02	0.00	c0.12	0.20
v/s Ratio Perm						0.00						
v/c Ratio	0.92	0.55	0.42	0.52	0.41	0.01	0.68	0.56	0.06	0.01	0.64	0.46
Uniform Delay, d1	49.2	35.8	15.4	56.3	46.6	43.3	46.3	45.0	32.8	42.1	47.7	25.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	27.2	1.0	0.6	3.4	0.9	0.0	2.5	1.7	0.1	0.0	5.2	1.0
Delay (s)	76.4	36.8	16.0	59.7	47.5	43.3	48.8	46.7	32.9	42.2	52.8	26.1
Level of Service	E	D	B	E	D	D	D	D	C	D	D	C
Approach Delay (s)		38.8			49.8			46.5			35.7	
Approach LOS		D			D			D			D	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			41.3				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			0.72									
Actuated Cycle Length (s)			127.0				Sum of lost time (s)			28.1		
Intersection Capacity Utilization			74.7%				ICU Level of Service			D		
Analysis Period (min)			15									

c Critical Lane Group

Queues

33: Hermitage Road & Dumbarton Road

10/12/2021



Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	1255	617	85	176
v/c Ratio	0.61	0.29	0.19	0.34
Control Delay	9.5	6.2	13.2	10.5
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	9.5	6.2	13.2	10.5
Queue Length 50th (ft)	137	48	15	20
Queue Length 95th (ft)	203	74	43	61
Internal Link Dist (ft)	196	212	242	190
Turn Bay Length (ft)				
Base Capacity (vph)	2048	2128	614	673
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.61	0.29	0.14	0.26
<b>Intersection Summary</b>				

# HCM Signalized Intersection Capacity Analysis

## 33: Hermitage Road & Dumbarton Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	72	1098	22	15	543	28	31	28	22	43	34	90
Future Volume (vph)	72	1098	22	15	543	28	31	28	22	43	34	90
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.5			4.5			4.5			4.5	
Lane Util. Factor		0.95			0.95			1.00			1.00	
Frb, ped/bikes		1.00			1.00			1.00			0.99	
Flpb, ped/bikes		1.00			1.00			1.00			1.00	
Frt		1.00			0.99			0.96			0.93	
Flt Protected		1.00			1.00			0.98			0.99	
Satd. Flow (prot)		3555			3544			1719			1722	
Flt Permitted		0.88			0.92			0.87			0.90	
Satd. Flow (perm)		3133			3251			1530			1568	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	76	1156	23	16	572	29	33	29	23	45	36	95
RTOR Reduction (vph)	0	2	0	0	5	0	0	18	0	0	75	0
Lane Group Flow (vph)	0	1253	0	0	612	0	0	67	0	0	101	0
Confl. Peds. (#/hr)	1		2	2		1	8					8
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	11%	0%	0%	0%	0%	0%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			4			2			2	
Permitted Phases	4			4			2			2		
Actuated Green, G (s)		33.4			33.4			11.4			11.4	
Effective Green, g (s)		33.4			33.4			11.4			11.4	
Actuated g/C Ratio		0.62			0.62			0.21			0.21	
Clearance Time (s)		4.5			4.5			4.5			4.5	
Vehicle Extension (s)		4.0			4.0			3.0			3.0	
Lane Grp Cap (vph)		1945			2018			324			332	
v/s Ratio Prot												
v/s Ratio Perm		c0.40			0.19			0.04			c0.06	
v/c Ratio		0.64			0.30			0.21			0.30	
Uniform Delay, d1		6.4			4.8			17.5			17.9	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.8			0.1			0.3			0.5	
Delay (s)		7.3			4.9			17.8			18.4	
Level of Service		A			A			B			B	
Approach Delay (s)		7.3			4.9			17.8			18.4	
Approach LOS		A			A			B			B	

### Intersection Summary

HCM 2000 Control Delay	7.9	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.56		
Actuated Cycle Length (s)	53.8	Sum of lost time (s)	9.0
Intersection Capacity Utilization	74.2%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

Queues

34: Lakeside Avenue & Dumbarton Road

10/12/2021



Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	896	320	480	168	159	622	185	585
v/c Ratio	0.98	0.53	0.70	0.38	1.34	0.71	1.34	0.65
Control Delay	64.0	10.6	42.7	7.8	236.2	37.7	229.8	36.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	10.6	42.7	7.8	236.2	37.7	229.8	36.0
Queue Length 50th (ft)	296	25	150	0	~132	176	~153	165
Queue Length 95th (ft)	#475	110	203	52	#276	257	#308	242
Internal Link Dist (ft)	371		350			338		383
Turn Bay Length (ft)		195		300	210		265	
Base Capacity (vph)	911	600	919	539	119	917	138	944
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.98	0.53	0.52	0.31	1.34	0.68	1.34	0.62

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.























# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 34: Lakeside Avenue & Dumbarton Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	100	733	298	73	374	156	148	450	128	172	461	83
Future Volume (vph)	100	733	298	73	374	156	148	450	128	172	461	83
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.5	5.5		5.5	5.5	5.5	6.0		5.5	6.0	
Lane Util. Factor		0.95	1.00		0.95	1.00	1.00	0.95		1.00	0.95	
Frbp, ped/bikes		1.00	0.99		1.00	1.00	1.00	1.00		1.00	1.00	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	0.97		1.00	0.98	
Flt Protected		0.99	1.00		0.99	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		3549	1563		3581	1615	1787	3478		1805	3489	
Flt Permitted		0.99	1.00		0.99	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)		3549	1563		3581	1615	1787	3478		1805	3489	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	108	788	320	78	402	168	159	484	138	185	496	89
RTOR Reduction (vph)	0	0	199	0	0	136	0	25	0	0	14	0
Lane Group Flow (vph)	0	896	121	0	480	32	159	597	0	185	571	0
Confl. Peds. (#/hr)			1	1			4		3	3		4
Heavy Vehicles (%)	2%	1%	2%	0%	0%	0%	1%	0%	0%	0%	1%	0%
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases			4			3						
Actuated Green, G (s)		25.1	25.1		18.8	18.8	6.5	23.8		7.5	24.8	
Effective Green, g (s)		25.1	25.1		18.8	18.8	6.5	23.8		7.5	24.8	
Actuated g/C Ratio		0.26	0.26		0.19	0.19	0.07	0.24		0.08	0.25	
Clearance Time (s)		5.5	5.5		5.5	5.5	5.5	6.0		5.5	6.0	
Vehicle Extension (s)		2.5	2.5		2.5	2.5	3.0	6.0		3.0	6.0	
Lane Grp Cap (vph)		911	401		689	310	118	847		138	885	
v/s Ratio Prot		c0.25			c0.13		0.09	c0.17		c0.10	0.16	
v/s Ratio Perm			0.08			0.02						
v/c Ratio		0.98	0.30		0.70	0.10	1.35	0.70		1.34	0.64	
Uniform Delay, d1		36.1	29.2		36.8	32.5	45.6	33.7		45.1	32.5	
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2		25.6	0.3		2.8	0.1	202.1	3.9		193.9	2.6	
Delay (s)		61.7	29.5		39.6	32.6	247.7	37.7		239.0	35.2	
Level of Service		E	C		D	C	F	D		F	D	
Approach Delay (s)		53.3			37.8			80.4			84.1	
Approach LOS		D			D			F			F	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			63.5				HCM 2000 Level of Service				E	
HCM 2000 Volume to Capacity ratio			0.86									
Actuated Cycle Length (s)			97.7				Sum of lost time (s)				22.5	
Intersection Capacity Utilization			80.9%				ICU Level of Service				D	
Analysis Period (min)			15									

c Critical Lane Group



Queues

12: Staples Mill Road & E Parham Road

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	160	597	167	303	652	262	280	1117	265	278	1256
v/c Ratio	0.45	0.82	0.25	0.74	0.83	0.37	0.68	1.05	0.36	0.68	0.83
Control Delay	38.3	42.0	7.9	47.3	42.4	11.7	43.7	72.0	9.4	43.6	32.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.3	42.0	7.9	47.3	42.4	11.7	43.7	72.0	9.4	43.6	32.4
Queue Length 50th (ft)	40	155	19	78	170	51	72	~338	46	71	218
Queue Length 95th (ft)	68	#238	59	#130	#274	111	111	#462	94	110	272
Internal Link Dist (ft)		473			499			459			344
Turn Bay Length (ft)	280		470	200		390	320			240	
Base Capacity (vph)	427	730	687	418	781	724	427	1063	733	427	1520
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.82	0.24	0.72	0.83	0.36	0.66	1.05	0.36	0.65	0.83

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
































Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 12: Staples Mill Road & E Parham Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 		 	 		 	 		 	 	
Traffic Volume (veh/h)	150	561	157	285	613	246	263	1050	249	261	1099	82
Future Volume (veh/h)	150	561	157	285	613	246	263	1050	249	261	1099	82
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1885	1870	1870	1870	1900	1900	1885	1870	1900	1885	1885
Adj Flow Rate, veh/h	160	597	0	303	652	262	280	1117	0	278	1169	87
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	1	2	2	2	0	0	1	2	0	1	1
Cap, veh/h	250	702		383	838	545	363	1174		362	1599	119
Arrive On Green	0.07	0.20	0.00	0.11	0.24	0.24	0.10	0.33	0.00	0.10	0.33	0.33
Sat Flow, veh/h	3510	3582	1585	3456	3554	1610	3510	3582	1585	3510	4887	364
Grp Volume(v), veh/h	160	597	0	303	652	262	280	1117	0	278	821	435
Grp Sat Flow(s),veh/h/ln	1755	1791	1585	1728	1777	1610	1755	1791	1585	1755	1716	1819
Q Serve(g_s), s	3.6	13.2	0.0	7.0	14.1	10.5	6.4	25.0	0.0	6.3	17.3	17.4
Cycle Q Clear(g_c), s	3.6	13.2	0.0	7.0	14.1	10.5	6.4	25.0	0.0	6.3	17.3	17.4
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.20
Lane Grp Cap(c), veh/h	250	702		383	838	545	363	1174		362	1123	595
V/C Ratio(X)	0.64	0.85		0.79	0.78	0.48	0.77	0.95		0.77	0.73	0.73
Avail Cap(c_a), veh/h	428	721		421	838	545	428	1174		428	1123	595
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.1	31.8	0.0	35.5	29.3	21.4	35.8	26.9	0.0	35.8	24.4	24.4
Incr Delay (d2), s/veh	2.0	12.3	0.0	8.6	7.0	3.0	6.5	16.9	0.0	6.4	4.2	7.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.6	6.7	0.0	3.3	6.6	4.3	3.0	12.9	0.0	3.0	7.4	8.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	39.1	44.1	0.0	44.1	36.4	24.4	42.3	43.8	0.0	42.2	28.6	32.1
LnGrp LOS	D	D		D	D	C	D	D		D	C	C
Approach Vol, veh/h		757	A		1217			1397	A		1534	
Approach Delay, s/veh		43.0			35.7			43.5			32.1	
Approach LOS		D			D			D			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.4	32.9	10.8	24.8	13.5	32.8	14.1	21.6				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.5	5.0	6.0	5.0	5.5				
Max Green Setting (Gmax), s	10.0	24.0	10.0	16.5	10.0	24.0	10.0	16.5				
Max Q Clear Time (g_c+I1), s	8.3	27.0	5.6	16.1	8.4	19.4	9.0	15.2				
Green Ext Time (p_c), s	0.1	0.0	0.1	0.4	0.1	4.4	0.1	0.9				

Intersection Summary

HCM 6th Ctrl Delay	37.9
HCM 6th LOS	D

Notes

Unsignalized Delay for [NBR, EBR] is excluded from calculations of the approach delay and intersection delay.

Queues

13: Staples Mill Road & Hermitage Road / Commercial Ent.

10/12/2021
























Lane Group	EBT	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	24	116	166	20	1477	113	151	1641
v/c Ratio	0.15	0.56	0.28	0.14	0.57	0.13	0.69	0.51
Control Delay	30.6	45.8	5.3	37.3	16.4	1.4	52.7	11.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	30.6	45.8	5.3	37.3	16.4	1.4	52.7	11.6
Queue Length 50th (ft)	9	56	0	10	182	0	72	101
Queue Length 95th (ft)	31	#123	43	31	266	14	#172	293
Internal Link Dist (ft)	192	548			450			520
Turn Bay Length (ft)			225	315		395	165	
Base Capacity (vph)	221	206	599	154	2588	860	222	3229
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.56	0.28	0.13	0.57	0.13	0.68	0.51

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
 13: Staples Mill Road & Hermitage Road / Commercial Ent.

10/12/2021

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Volume (vph)	16	1	6	108	1	156	19	1388	106	142	1529	13	
Future Volume (vph)	16	1	6	108	1	156	19	1388	106	142	1529	13	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0		
Lane Util. Factor		1.00			1.00	1.00	1.00	0.91	1.00	1.00	0.91		
Frbp, ped/bikes		1.00			1.00	1.00	1.00	1.00	0.98	1.00	1.00		
Flpb, ped/bikes		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Frt		0.97			1.00	0.85	1.00	1.00	0.85	1.00	1.00		
Flt Protected		0.97			0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (prot)		1773			1775	1599	1805	5136	1550	1787	5129		
Flt Permitted		0.97			0.95	1.00	0.95	1.00	1.00	0.95	1.00		
Satd. Flow (perm)		1773			1775	1599	1805	5136	1550	1787	5129		
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	17	1	6	115	1	166	20	1477	113	151	1627	14	
RTOR Reduction (vph)	0	6	0	0	0	126	0	0	60	0	1	0	
Lane Group Flow (vph)	0	18	0	0	116	40	20	1477	53	151	1640	0	
Confl. Peds. (#/hr)							2		1	1		2	
Heavy Vehicles (%)	0%	0%	0%	2%	0%	1%	0%	1%	2%	1%	1%	0%	
Turn Type	Split	NA		Split	NA	pm+ov	Prot	NA	Perm	Prot	NA		
Protected Phases	3	3		4	4	1	5	2		1	6		
Permitted Phases						4			2				
Actuated Green, G (s)		3.0			9.6	19.7	2.8	38.3	38.3	10.1	45.6		
Effective Green, g (s)		3.0			9.6	19.7	2.8	38.3	38.3	10.1	45.6		
Actuated g/C Ratio		0.04			0.12	0.24	0.03	0.47	0.47	0.12	0.56		
Clearance Time (s)		5.0			5.0	5.0	5.0	6.0	6.0	5.0	6.0		
Vehicle Extension (s)		3.0			3.0	3.0	3.0	5.0	5.0	3.0	5.0		
Lane Grp Cap (vph)		64			207	481	61	2398	723	220	2852		
v/s Ratio Prot		c0.01			c0.07	0.01	0.01	c0.29		c0.08	0.32		
v/s Ratio Perm						0.01			0.03				
v/c Ratio		0.28			0.56	0.08	0.33	0.62	0.07	0.69	0.58		
Uniform Delay, d1		38.5			34.2	24.1	38.7	16.3	12.1	34.4	11.9		
Progression Factor		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2		2.4			3.4	0.1	3.1	1.2	0.2	8.6	0.9		
Delay (s)		40.9			37.6	24.2	41.8	17.5	12.3	43.0	12.7		
Level of Service		D			D	C	D	B	B	D	B		
Approach Delay (s)		40.9			29.7			17.5			15.3		
Approach LOS		D			C			B			B		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			17.5									HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio			0.60										
Actuated Cycle Length (s)			82.0									Sum of lost time (s)	21.0
Intersection Capacity Utilization			56.1%									ICU Level of Service	B
Analysis Period (min)			15										

c Critical Lane Group

Queues

14: Staples Mill Road & Wistar Road

10/12/2021



Lane Group	EBT	EBR	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	129	164	28	133	1405	13	1459	128
v/c Ratio	0.71	0.46	0.13	0.64	0.40	0.08	0.53	0.14
Control Delay	55.7	10.1	21.1	48.7	6.8	35.8	13.7	2.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.7	10.1	21.1	48.7	6.8	35.8	13.7	2.5
Queue Length 50th (ft)	64	0	6	65	88	6	176	0
Queue Length 95th (ft)	#141	53	29	#124	189	23	217	25
Internal Link Dist (ft)	312		290		483		269	
Turn Bay Length (ft)		215		415		275		1000
Base Capacity (vph)	196	374	238	228	3485	187	2737	911
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.66	0.44	0.12	0.58	0.40	0.07	0.53	0.14






















Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 14: Staples Mill Road & Wistar Road

10/12/2021

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	127	1	162	9	4	15	132	1379	12	13	1444	127
Future Volume (vph)	127	1	162	9	4	15	132	1379	12	13	1444	127
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0		6.0		5.5	6.0		5.5	6.0	6.0
Lane Util. Factor		1.00	1.00		1.00		1.00	0.91		1.00	0.91	1.00
Frt		1.00	0.85		0.93		1.00	1.00		1.00	1.00	0.85
Flt Protected		0.95	1.00		0.98		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)		1810	1599		1735		1787	5080		1805	5136	1599
Flt Permitted		0.71	1.00		0.87		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)		1344	1599		1540		1787	5080		1805	5136	1599
Peak-hour factor, PHF	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Adj. Flow (vph)	128	1	164	9	4	15	133	1393	12	13	1459	128
RTOR Reduction (vph)	0	0	142	0	13	0	0	1	0	0	0	60
Lane Group Flow (vph)	0	129	22	0	15	0	133	1404	0	13	1459	68
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	1%	2%	0%	0%	1%	1%
Turn Type	Perm	NA	Perm	Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4			4		5	2		1	6	
Permitted Phases	4		4	4								6
Actuated Green, G (s)		11.2	11.2		11.2		9.6	51.8		1.5	43.7	43.7
Effective Green, g (s)		11.2	11.2		11.2		9.6	51.8		1.5	43.7	43.7
Actuated g/C Ratio		0.14	0.14		0.14		0.12	0.63		0.02	0.53	0.53
Clearance Time (s)		6.0	6.0		6.0		5.5	6.0		5.5	6.0	6.0
Vehicle Extension (s)		3.5	3.5		3.5		3.0	5.0		3.0	5.0	5.0
Lane Grp Cap (vph)		183	218		210		209	3209		33	2737	852
v/s Ratio Prot							c0.07	0.28		0.01	c0.28	
v/s Ratio Perm		c0.10	0.01		0.01							0.04
v/c Ratio		0.70	0.10		0.07		0.64	0.44		0.39	0.53	0.08
Uniform Delay, d1		33.8	31.0		30.9		34.5	7.7		39.8	12.5	9.3
Progression Factor		1.00	1.00		1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2		12.1	0.2		0.2		6.2	0.4		7.6	0.7	0.2
Delay (s)		45.9	31.2		31.0		40.7	8.1		47.4	13.2	9.5
Level of Service		D	C		C		D	A		D	B	A
Approach Delay (s)		37.7			31.0			10.9			13.2	
Approach LOS		D			C			B			B	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			14.4				HCM 2000 Level of Service				B	
HCM 2000 Volume to Capacity ratio			0.58									
Actuated Cycle Length (s)			82.0				Sum of lost time (s)			17.5		
Intersection Capacity Utilization			63.6%				ICU Level of Service			B		
Analysis Period (min)			15									
c Critical Lane Group												

Queues

15: Staples Mill Road & Bremner Boulevard

10/12/2021



Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	86	105	65	16	85	1557	56	1551
v/c Ratio	0.53	0.35	0.44	0.06	0.47	0.51	0.30	0.54
Control Delay	47.7	4.4	45.2	0.4	42.9	14.4	38.2	16.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.7	4.4	45.2	0.4	42.9	14.4	38.2	16.2
Queue Length 50th (ft)	43	0	32	0	42	216	27	217
Queue Length 95th (ft)	88	11	72	0	84	279	61	277
Internal Link Dist (ft)	178		254			316		213
Turn Bay Length (ft)		115		100	315		320	
Base Capacity (vph)	172	310	156	285	220	3059	220	2863
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.50	0.34	0.42	0.06	0.39	0.51	0.25	0.54

Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 15: Staples Mill Road & Bremner Boulevard

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕	↗		↕	↗	↗	↑↑↑		↗	↑↑↑		
Traffic Volume (vph)	68	17	103	49	15	16	83	1518	8	55	1459	61	
Future Volume (vph)	68	17	103	49	15	16	83	1518	8	55	1459	61	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0		
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	0.91		1.00	0.91		
Frbp, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00		
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00		
Frt		1.00	0.85		1.00	0.85	1.00	1.00		1.00	0.99		
Flt Protected		0.96	1.00		0.96	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (prot)		1770	1583		1830	1495	1805	5131		1805	5095		
Flt Permitted		0.96	1.00		0.96	1.00	0.95	1.00		0.95	1.00		
Satd. Flow (perm)		1770	1583		1830	1495	1805	5131		1805	5095		
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	
Adj. Flow (vph)	69	17	105	50	15	16	85	1549	8	56	1489	62	
RTOR Reduction (vph)	0	0	97	0	0	15	0	0	0	0	5	0	
Lane Group Flow (vph)	0	86	8	0	65	1	85	1557	0	56	1546	0	
Confl. Peds. (#/hr)							1		1	1		1	
Heavy Vehicles (%)	4%	0%	2%	0%	0%	8%	0%	1%	0%	0%	1%	4%	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA		Prot	NA		
Protected Phases	4	4		3	3		5	2		1	6		
Permitted Phases			4			3							
Actuated Green, G (s)		6.4	6.4		5.6	5.6	7.2	43.7		5.3	41.8		
Effective Green, g (s)		6.4	6.4		5.6	5.6	7.2	43.7		5.3	41.8		
Actuated g/C Ratio		0.08	0.08		0.07	0.07	0.09	0.53		0.06	0.51		
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0		
Vehicle Extension (s)		3.0	3.0		3.0	3.0	2.5	6.0		2.5	6.0		
Lane Grp Cap (vph)		138	123		124	102	158	2734		116	2597		
v/s Ratio Prot		c0.05			c0.04		c0.05	0.30		0.03	c0.30		
v/s Ratio Perm			0.01			0.00							
v/c Ratio		0.62	0.07		0.52	0.01	0.54	0.57		0.48	0.60		
Uniform Delay, d1		36.6	35.0		36.9	35.6	35.8	12.8		37.0	14.1		
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00		
Incremental Delay, d2		8.5	0.2		4.0	0.0	2.7	0.9		2.3	1.0		
Delay (s)		45.1	35.3		40.9	35.7	38.5	13.7		39.3	15.2		
Level of Service		D	D		D	D	D	B		D	B		
Approach Delay (s)		39.7			39.8			15.0			16.0		
Approach LOS		D			D			B			B		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			17.4		HCM 2000 Level of Service						B		
HCM 2000 Volume to Capacity ratio			0.58										
Actuated Cycle Length (s)			82.0		Sum of lost time (s)					21.0			
Intersection Capacity Utilization			60.8%		ICU Level of Service					B			
Analysis Period (min)			15										

c Critical Lane Group



Queues

16: Staples Mill Road & Amtrak Station

10/12/2021



Lane Group	WBL	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	27	10	1765	10	1734
v/c Ratio	0.16	0.06	0.42	0.06	0.40
Control Delay	36.2	19.3	4.9	35.6	2.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	36.2	19.3	4.9	35.6	2.9
Queue Length 50th (ft)	13	0	91	5	89
Queue Length 95th (ft)	37	14	243	20	124
Internal Link Dist (ft)	469		187		179
Turn Bay Length (ft)				170	
Base Capacity (vph)	398	364	4188	158	4375
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.07	0.03	0.42	0.06	0.40

Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 16: Staples Mill Road & Amtrak Station

10/12/2021



Movement	WBL	WBR	NBU	NBT	NBR	SBL	SBT
Lane Configurations	↙	↗	↘	↑↑↑		↙	↑↑↑
Traffic Volume (vph)	25	9	0	1610	32	9	1613
Future Volume (vph)	25	9	0	1610	32	9	1613
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.3	6.3		5.9		6.8	5.9
Lane Util. Factor	1.00	1.00		0.91		1.00	0.91
Frbp, ped/bikes	1.00	1.00		1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00		1.00		1.00	1.00
Frt	1.00	0.85		1.00		1.00	1.00
Flt Protected	0.95	1.00		1.00		0.95	1.00
Satd. Flow (prot)	1805	1615		5120		1805	5136
Flt Permitted	0.95	1.00		1.00		0.95	1.00
Satd. Flow (perm)	1805	1615		5120		1805	5136
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	27	10	0	1731	34	10	1734
RTOR Reduction (vph)	0	9	0	2	0	0	0
Lane Group Flow (vph)	27	1	0	1763	0	10	1734
Confl. Peds. (#/hr)					1	1	
Heavy Vehicles (%)	0%	0%	2%	1%	0%	0%	1%
Turn Type	Prot	Perm	Prot	NA		Prot	NA
Protected Phases	4		5	2		1	6
Permitted Phases		4					
Actuated Green, G (s)	4.8	4.8		56.8		1.4	65.0
Effective Green, g (s)	4.8	4.8		56.8		1.4	65.0
Actuated g/C Ratio	0.06	0.06		0.69		0.02	0.79
Clearance Time (s)	6.3	6.3		5.9		6.8	5.9
Vehicle Extension (s)	3.5	3.5		5.5		3.0	5.5
Lane Grp Cap (vph)	105	94		3546		30	4071
v/s Ratio Prot	c0.01			c0.34		0.01	c0.34
v/s Ratio Perm		0.00					
v/c Ratio	0.26	0.01		0.50		0.33	0.43
Uniform Delay, d1	36.9	36.4		5.9		39.8	2.7
Progression Factor	1.00	1.00		1.00		1.00	1.00
Incremental Delay, d2	1.5	0.0		0.5		6.5	0.3
Delay (s)	38.4	36.4		6.4		46.3	3.0
Level of Service	D	D		A		D	A
Approach Delay (s)	37.9			6.4			3.2
Approach LOS	D			A			A

### Intersection Summary

HCM 2000 Control Delay	5.2	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.51		
Actuated Cycle Length (s)	82.0	Sum of lost time (s)	19.0
Intersection Capacity Utilization	47.8%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

Queues

17: Staples Mill Road & Crockett Street

10/12/2021



Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	139	70	62	43	121	1599	24	50	1613	88
v/c Ratio	0.66	0.21	0.39	0.14	0.60	0.59	0.03	0.32	0.66	0.11
Control Delay	50.3	1.5	41.9	1.0	48.1	18.1	0.0	40.9	21.5	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.3	1.5	41.9	1.0	48.1	18.1	0.0	40.9	21.5	0.7
Queue Length 50th (ft)	70	0	31	0	61	243	0	25	265	0
Queue Length 95th (ft)	#129	0	67	0	#117	339	0	57	343	5
Internal Link Dist (ft)	284		261			193			145	
Turn Bay Length (ft)		115			190		75	295		105
Base Capacity (vph)	237	350	223	354	222	2695	910	208	2445	834
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.59	0.20	0.28	0.12	0.55	0.59	0.03	0.24	0.66	0.11

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 17: Staples Mill Road & Crockett Street

10/12/2021



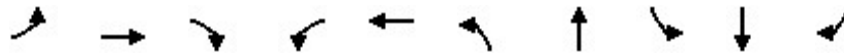
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕	↗		↕	↗	↗	↑↑↑	↗	↗	↑↑↑	↗	
Traffic Volume (vph)	124	7	66	53	6	40	114	1503	23	47	1516	83	
Future Volume (vph)	124	7	66	53	6	40	114	1503	23	47	1516	83	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.0		5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0	
Lane Util. Factor		1.00	1.00		1.00	1.00	1.00	0.91	1.00	1.00	0.91	1.00	
Frbp, ped/bikes		1.00	1.00		1.00	0.98	1.00	1.00	0.99	1.00	1.00	0.98	
Flpb, ped/bikes		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt		1.00	0.85		1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected		0.95	1.00		0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)		1814	1553		1710	1584	1787	5136	1595	1752	5136	1581	
Flt Permitted		0.95	1.00		0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (perm)		1814	1553		1710	1584	1787	5136	1595	1752	5136	1581	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	132	7	70	56	6	43	121	1599	24	50	1613	88	
RTOR Reduction (vph)	0	0	62	0	0	39	0	0	12	0	0	48	
Lane Group Flow (vph)	0	139	8	0	62	4	121	1599	12	50	1613	40	
Confl. Peds. (#/hr)	2					2	1		1	1		1	
Heavy Vehicles (%)	0%	0%	4%	7%	0%	0%	1%	1%	0%	3%	1%	0%	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	
Protected Phases	3	3		4	4		5	2		1	6		
Permitted Phases			3			4			2			6	
Actuated Green, G (s)		9.8	9.8		6.9	6.9	8.2	41.2	41.2	5.1	38.1	38.1	
Effective Green, g (s)		9.8	9.8		6.9	6.9	8.2	41.2	41.2	5.1	38.1	38.1	
Actuated g/C Ratio		0.12	0.12		0.08	0.08	0.10	0.49	0.49	0.06	0.45	0.45	
Clearance Time (s)		5.0	5.0		5.0	5.0	5.0	6.0	6.0	5.0	6.0	6.0	
Vehicle Extension (s)		2.5	2.5		2.5	2.5	2.5	4.0	4.0	2.5	4.0	4.0	
Lane Grp Cap (vph)		211	181		140	130	174	2519	782	106	2329	717	
v/s Ratio Prot		c0.08			c0.04		c0.07	c0.31		0.03	c0.31		
v/s Ratio Perm			0.01			0.00			0.01			0.03	
v/c Ratio		0.66	0.05		0.44	0.03	0.70	0.63	0.02	0.47	0.69	0.06	
Uniform Delay, d1		35.5	32.9		36.7	35.5	36.7	15.8	11.0	38.1	18.3	12.9	
Progression Factor		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2		6.5	0.1		1.6	0.1	10.6	1.2	0.0	2.4	1.7	0.1	
Delay (s)		42.0	33.0		38.3	35.5	47.3	17.1	11.0	40.5	20.0	13.0	
Level of Service		D	C		D	D	D	B	B	D	C	B	
Approach Delay (s)		39.0			37.2			19.1			20.2		
Approach LOS		D			D			B			C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay			21.2		HCM 2000 Level of Service						C		
HCM 2000 Volume to Capacity ratio			0.66										
Actuated Cycle Length (s)			84.0		Sum of lost time (s)						21.0		
Intersection Capacity Utilization			62.8%		ICU Level of Service						B		
Analysis Period (min)			15										

c Critical Lane Group

Queues

18: Staples Mill Road & Hilliard Road / Glenside Drive

10/12/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	183	331	120	135	602	156	1434	324	1185	238
v/c Ratio	0.93	0.63	0.32	0.77	1.06	0.67	0.89	0.98	0.62	0.33
Control Delay	88.5	39.0	4.3	65.5	84.3	48.6	34.3	81.4	23.3	4.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	88.5	39.0	4.3	65.5	84.3	48.6	34.3	81.4	23.3	4.1
Queue Length 50th (ft)	95	85	0	69	~148	76	247	167	183	0
Queue Length 95th (ft)	#216	128	19	#160	#254	#142	#334	#328	230	45
Internal Link Dist (ft)		762			867		276		418	
Turn Bay Length (ft)	200		215	725		280		330		245
Base Capacity (vph)	196	524	376	176	567	256	1611	330	1905	724
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.93	0.63	0.32	0.77	1.06	0.61	0.89	0.98	0.62	0.33

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM 6th Signalized Intersection Summary  
 18: Staples Mill Road & Hilliard Road / Glenside Drive

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑		↘	↑↑↑		↘	↑↑↑	↗
Traffic Volume (veh/h)	176	318	115	130	360	218	150	1177	200	311	1138	228
Future Volume (veh/h)	176	318	115	130	360	218	150	1177	200	311	1138	228
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1885	1885	1870	1900	1900	1900	1856	1885	1885	1900	1900	1870
Adj Flow Rate, veh/h	183	331	120	135	375	227	156	1226	208	324	1185	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	1	1	2	0	0	0	3	1	1	0	0	2
Cap, veh/h	197	540	239	168	291	174	192	1404	238	331	2029	
Arrive On Green	0.11	0.15	0.15	0.09	0.13	0.13	0.11	0.32	0.32	0.18	0.39	0.00
Sat Flow, veh/h	1795	3582	1582	1810	2173	1296	1767	4428	751	1810	5187	1585
Grp Volume(v), veh/h	183	331	120	135	311	291	156	950	484	324	1185	0
Grp Sat Flow(s),veh/h/ln	1795	1791	1582	1810	1805	1663	1767	1716	1748	1810	1729	1585
Q Serve(g_s), s	8.3	7.1	5.7	6.0	11.0	11.0	7.1	21.4	21.4	14.6	14.8	0.0
Cycle Q Clear(g_c), s	8.3	7.1	5.7	6.0	11.0	11.0	7.1	21.4	21.4	14.6	14.8	0.0
Prop In Lane	1.00		1.00	1.00		0.78	1.00		0.43	1.00		1.00
Lane Grp Cap(c), veh/h	197	540	239	168	242	223	192	1088	554	331	2029	
V/C Ratio(X)	0.93	0.61	0.50	0.80	1.28	1.31	0.81	0.87	0.87	0.98	0.58	
Avail Cap(c_a), veh/h	197	540	239	177	242	223	259	1088	554	331	2029	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	36.2	32.6	32.0	36.4	35.5	35.5	35.7	26.4	26.4	33.3	19.7	0.0
Incr Delay (d2), s/veh	43.7	2.0	1.7	21.9	154.8	166.3	13.2	9.7	17.2	43.6	1.2	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.9	3.2	2.2	3.6	15.1	14.6	3.7	9.8	11.1	10.2	5.9	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	79.9	34.6	33.7	58.4	190.3	201.8	48.9	36.2	43.6	76.9	20.9	0.0
LnGrp LOS	E	C	C	E	F	F	D	D	D	E	C	
Approach Vol, veh/h		634			737			1590			1509	A
Approach Delay, s/veh		47.5			170.7			39.7			33.0	
Approach LOS		D			F			D			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	20.0	32.0	12.6	17.4	13.9	38.1	14.0	16.0				
Change Period (Y+Rc), s	5.0	6.0	5.0	5.0	5.0	6.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	26.0	8.0	12.0	12.0	29.0	9.0	11.0				
Max Q Clear Time (g_c+I1), s	16.6	23.4	8.0	9.1	9.1	16.8	10.3	13.0				
Green Ext Time (p_c), s	0.0	2.2	0.0	0.7	0.1	7.9	0.0	0.0				

Intersection Summary

HCM 6th Ctrl Delay	60.1
HCM 6th LOS	E

Notes

Unsignalized Delay for [SBR] is excluded from calculations of the approach delay and intersection delay.

Queues

19: Staples Mill Road & Aspen Avenue / Townhouse Road

10/12/2021



Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	123	53	51	1520	21	1416
v/c Ratio	0.60	0.35	0.17	0.44	0.08	0.42
Control Delay	29.3	36.8	6.4	9.5	5.8	10.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.3	36.8	6.4	9.5	5.8	10.8
Queue Length 50th (ft)	23	21	8	132	3	172
Queue Length 95th (ft)	#78	56	20	236	11	220
Internal Link Dist (ft)	218	764		385		854
Turn Bay Length (ft)			230		215	
Base Capacity (vph)	215	162	316	3482	312	3358
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.57	0.33	0.16	0.44	0.07	0.42

Intersection Summary

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

# HCM Signalized Intersection Capacity Analysis

## 19: Staples Mill Road & Aspen Avenue / Townhouse Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↑↑↑		↕	↑↑↑	
Traffic Volume (vph)	40	5	73	32	10	10	49	1428	31	20	1319	40
Future Volume (vph)	40	5	73	32	10	10	49	1428	31	20	1319	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0			5.0		5.0	6.0		5.0	6.0	
Lane Util. Factor		1.00			1.00		1.00	0.91		1.00	0.91	
Frbp, ped/bikes		1.00			1.00		1.00	1.00		1.00	1.00	
Flpb, ped/bikes		1.00			1.00		1.00	1.00		1.00	1.00	
Frt		0.92			0.97		1.00	1.00		1.00	1.00	
Flt Protected		0.98			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1712			1796		1752	5067		1805	5111	
Flt Permitted		0.98			0.97		0.14	1.00		0.13	1.00	
Satd. Flow (perm)		1712			1796		264	5067		256	5111	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	42	5	76	33	10	10	51	1488	32	21	1374	42
RTOR Reduction (vph)	0	71	0	0	9	0	0	2	0	0	3	0
Lane Group Flow (vph)	0	52	0	0	44	0	51	1518	0	21	1413	0
Confl. Peds. (#/hr)							5					5
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	3%	2%	4%	0%	1%	0%
Turn Type	Split	NA		Split	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases							2			6		
Actuated Green, G (s)		5.4			4.2		52.9	49.1		49.9	47.6	
Effective Green, g (s)		5.4			4.2		52.9	49.1		49.9	47.6	
Actuated g/C Ratio		0.07			0.05		0.65	0.60		0.61	0.58	
Clearance Time (s)		5.0			5.0		5.0	6.0		5.0	6.0	
Vehicle Extension (s)		3.0			3.0		2.5	4.0		2.5	4.0	
Lane Grp Cap (vph)		112			91		239	3034		199	2966	
v/s Ratio Prot		c0.03			c0.02		c0.01	c0.30		0.00	0.28	
v/s Ratio Perm							0.13			0.06		
v/c Ratio		0.46			0.48		0.21	0.50		0.11	0.48	
Uniform Delay, d1		36.9			37.8		6.0	9.4		6.7	10.0	
Progression Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2		3.0			3.9		0.3	0.6		0.2	0.6	
Delay (s)		39.9			41.8		6.3	10.0		6.9	10.5	
Level of Service		D			D		A	B		A	B	
Approach Delay (s)		39.9			41.8			9.9			10.5	
Approach LOS		D			D			A			B	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			11.8				HCM 2000 Level of Service			B		
HCM 2000 Volume to Capacity ratio			0.49									
Actuated Cycle Length (s)			82.0				Sum of lost time (s)			21.0		
Intersection Capacity Utilization			53.0%				ICU Level of Service			A		
Analysis Period (min)			15									
c Critical Lane Group												



Queues

20: Staples Mill Road & Dumbarton Road / Wharfside Road

10/12/2021

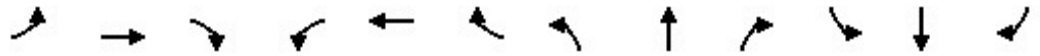


Lane Group	EBL	EBT	WBL	WBR	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	5	9	366	200	1345	448	197	1281
v/c Ratio	0.05	0.08	0.71	0.29	0.58	0.46	0.71	0.37
Control Delay	37.2	25.7	41.6	6.1	19.7	3.9	46.6	7.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	37.2	25.7	41.6	6.1	19.7	3.9	46.6	7.1
Queue Length 50th (ft)	3	1	91	15	171	0	96	79
Queue Length 95th (ft)	13	16	137	55	278	61	162	151
Internal Link Dist (ft)		123			241			150
Turn Bay Length (ft)				250		230	235	
Base Capacity (vph)	110	108	552	725	2304	969	330	3435
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.08	0.66	0.28	0.58	0.46	0.60	0.37

Intersection Summary

HCM Signalized Intersection Capacity Analysis  
 20: Staples Mill Road & Dumbarton Road / Wharfside Road

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖↗		↖		↑↑↑	↖	↖	↑↑↑	
Traffic Volume (vph)	5	2	7	359	0	196	0	1318	439	193	1255	0
Future Volume (vph)	5	2	7	359	0	196	0	1318	439	193	1255	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0		5.0		6.0	6.0	5.0	6.0	
Lane Util. Factor	1.00	1.00		0.97		1.00		0.91	1.00	1.00	0.91	
Frbp, ped/bikes	1.00	1.00		1.00		1.00		1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00		1.00		1.00		1.00	1.00	1.00	1.00	
Frt	1.00	0.88		1.00		0.85		1.00	0.85	1.00	1.00	
Flt Protected	0.95	1.00		0.95		1.00		1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1805	1678		3467		1615		5085	1599	1805	5136	
Flt Permitted	0.95	1.00		0.95		1.00		1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1805	1678		3467		1615		5085	1599	1805	5136	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	5	2	7	366	0	200	0	1345	448	197	1281	0
RTOR Reduction (vph)	0	7	0	0	0	107	0	0	261	0	0	0
Lane Group Flow (vph)	5	2	0	366	0	93	0	1345	187	197	1281	0
Confl. Peds. (#/hr)							3					3
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%	0%	2%	1%	0%	1%	0%
Turn Type	Split	NA		Prot		pm+ov		NA	Perm	Prot	NA	
Protected Phases	3	3		4		1		2		1	6	
Permitted Phases						4			2			
Actuated Green, G (s)	2.0	2.0		12.1		24.8		34.2	34.2	12.7	51.9	
Effective Green, g (s)	2.0	2.0		12.1		24.8		34.2	34.2	12.7	51.9	
Actuated g/C Ratio	0.02	0.02		0.15		0.30		0.42	0.42	0.15	0.63	
Clearance Time (s)	5.0	5.0		5.0		5.0		6.0	6.0	5.0	6.0	
Vehicle Extension (s)	2.5	2.5		2.5		2.5		6.0	6.0	2.5	6.0	
Lane Grp Cap (vph)	44	40		511		586		2120	666	279	3250	
v/s Ratio Prot	c0.00	0.00		c0.11		0.02		c0.26		c0.11	0.25	
v/s Ratio Perm						0.03			0.12			
v/c Ratio	0.11	0.05		0.72		0.16		0.63	0.28	0.71	0.39	
Uniform Delay, d1	39.1	39.1		33.3		21.0		18.9	15.8	32.9	7.4	
Progression Factor	1.00	1.00		1.00		1.00		1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.8	0.4		4.4		0.1		1.5	1.1	7.3	0.4	
Delay (s)	40.0	39.5		37.7		21.1		20.4	16.8	40.2	7.7	
Level of Service	D	D		D		C		C	B	D	A	
Approach Delay (s)		39.7			31.8			19.5			12.1	
Approach LOS		D			C			B			B	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			18.5									B
HCM 2000 Volume to Capacity ratio			0.65									
Actuated Cycle Length (s)			82.0							21.0		
Intersection Capacity Utilization			65.6%									C
Analysis Period (min)			15									
c Critical Lane Group												

Queues

21: Staples Mill Road & Dickens Road / Ent. to Comcast

10/12/2021



Lane Group	EBL	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	116	116	55	11	2	6	1577	5	1679
v/c Ratio	0.59	0.59	0.16	0.09	0.01	0.05	0.45	0.04	0.48
Control Delay	46.7	46.7	1.0	36.9	0.0	36.5	7.9	36.4	8.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.7	46.7	1.0	36.9	0.0	36.5	7.9	36.4	8.1
Queue Length 50th (ft)	60	60	0	5	0	3	90	3	97
Queue Length 95th (ft)	115	115	0	21	0	15	271	13	293
Internal Link Dist (ft)		91		125			466		240
Turn Bay Length (ft)			105		50	450		180	
Base Capacity (vph)	217	217	359	155	296	176	3524	176	3527
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.53	0.53	0.15	0.07	0.01	0.03	0.45	0.03	0.48

Intersection Summary

# HCM Signalized Intersection Capacity Analysis

## 21: Staples Mill Road & Dickens Road / Ent. to Comcast

10/12/2021



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖	↖		↖	↖	↖	↑↑↑		↖	↑↑↑	
Traffic Volume (vph)	225	0	53	9	2	2	6	1522	8	5	1502	127
Future Volume (vph)	225	0	53	9	2	2	6	1522	8	5	1502	127
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0	
Lane Util. Factor	0.95	0.95	1.00		1.00	1.00	1.00	0.91		1.00	0.91	
Frt	1.00	1.00	0.85		1.00	0.85	1.00	1.00		1.00	0.99	
Flt Protected	0.95	0.95	1.00		0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1681	1681	1615		1825	1615	1805	5082		1805	5079	
Flt Permitted	0.95	0.95	1.00		0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1681	1681	1615		1825	1615	1805	5082		1805	5079	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	232	0	55	9	2	2	6	1569	8	5	1548	131
RTOR Reduction (vph)	0	0	49	0	0	2	0	0	0	0	8	0
Lane Group Flow (vph)	116	116	6	0	11	0	6	1577	0	5	1671	0
Heavy Vehicles (%)	2%	0%	0%	0%	0%	0%	0%	2%	0%	0%	1%	0%
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	4	4		3	3		5	2		1		6
Permitted Phases			4			3						
Actuated Green, G (s)	9.6	9.6	9.6		1.3	1.3	1.2	48.9		1.2	48.9	
Effective Green, g (s)	9.6	9.6	9.6		1.3	1.3	1.2	48.9		1.2	48.9	
Actuated g/C Ratio	0.12	0.12	0.12		0.02	0.02	0.01	0.60		0.01	0.60	
Clearance Time (s)	5.0	5.0	5.0		5.0	5.0	5.0	6.0		5.0	6.0	
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5	2.5	6.0		2.5	6.0	
Lane Grp Cap (vph)	196	196	189		28	25	26	3030		26	3028	
v/s Ratio Prot	c0.07	0.07			c0.01		c0.00	0.31		0.00	c0.33	
v/s Ratio Perm			0.00			0.00						
v/c Ratio	0.59	0.59	0.03		0.39	0.00	0.23	0.52		0.19	0.55	
Uniform Delay, d1	34.3	34.3	32.1		40.0	39.7	39.9	9.7		39.9	10.0	
Progression Factor	1.00	1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	4.0	4.0	0.1		6.5	0.0	3.3	0.6		2.6	0.7	
Delay (s)	38.3	38.3	32.1		46.5	39.7	43.2	10.3		42.5	10.7	
Level of Service	D	D	C		D	D	D	B		D	B	
Approach Delay (s)		37.1			45.4			10.5			10.8	
Approach LOS		D			D			B			B	
<b>Intersection Summary</b>												
HCM 2000 Control Delay			12.9		HCM 2000 Level of Service						B	
HCM 2000 Volume to Capacity ratio			0.55									
Actuated Cycle Length (s)			82.0		Sum of lost time (s)					21.0		
Intersection Capacity Utilization			53.9%		ICU Level of Service					A		
Analysis Period (min)			15									
c Critical Lane Group												